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Knight Piés

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DATE:

Sept 2/99

FILE NO.:

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REF NO.:

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TIME:

**OPERATOR:** 

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PAGES:

1 of 10

SENDER:

John Wilkinson

APPROVED:

Ministry of Energy & Mines,

TO:

Mines Branch, Victoria

FAX:

Brian McBride, MEM, Prince George

Eric LeNeve, MPMC

ATTN:

George Headley

SUBJECT:

Trial Downstream Cycloned Sand Berm

cc:

Dear George,

Attached please find a letter from Knight Piésold to Eric LeNeve of Mt. Polley Mining Corporation regarding the proposed trial downstream cycloned sand berm, to be located along the perimeter embankment of the Tailings Storage Facility. The attached letter describes the purposes of the trial, the proposed arrangement of the berm, the type of information that is required from the trial, and the approximate costs.

Construction of the trial berm is scheduled to start late this week, and will likely continue for a few weeks, with piezometer readings, sampling, and CPT and laboratory testing continuing during and after the cycloning period. The purpose of this fax is to inform you of the trial berm arrangement and to invite you to visit the Mt. Polley Mine site at some point over the next two to three weeks to observe the performance of the berm and review any collected data. For your information, I am intending to be on site for most of the week of September 13th, and I would be pleased to visit the trial berm with you during this period.

Please contact our office if you have any questions regarding the trial or if you would like to coordinate our site visits any further.

Best Regards,

Knight Piésold Ltd.

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Suite 1400 750 West Pender Street Vancouver, British Columbia Canada V6C 2T8

Mr. Eric LeNeve
Mt. Polley Mining Corp
P.O. Box 12
Likely, B.C.
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Our Reference No: 11162/12.01

Number:

9/1883

July 7, 1999

Dear Eric:

# Re: Mount Polley Project Trial Downstream Cycloned Sand Berm

Knight Piésold Ltd. is pleased to provide the following plan of work for the proposed construction and monitoring of a trial berm consisting of cycloned tailings material at the Mount Polley Mine. The trial sand berm will be located on a section of the downstream face of the existing Stage 2B Perimeter Embankment of the Tailings Storage Facility (TSF). Information to be drawn from this trial study is needed to finalize design concepts and issues involved in the permitting and use of cycloned tailings in on-going construction of the TSF embankments. The TSF is currently permitted and constructed to the Stage 2B elevation of 937 m.a.s.l.

### Background

The Knight Piésold Report titled 'Evaluation of Cycloned Tailings for Embankment Construction' (Ref. No. 11162/11-1, Rev. 0) provided a preliminary evaluation of the use of cycloned tailings sand for construction of the TSF embankments. Geotechnical characteristics of trial cyclone outputs illustrated that the sand is well-suited for embankment fill. A Preferred Option was chosen for use of cycloned sand as embankment fill which took into account material availability, geotechnical concerns and capital costs. This Preferred Option incorporates cycloned tailings both upstream and downstream of the retained core zone and involves construction of a confining berm along the downstream toe of the embankment, as well as a system of ditches to collect seepage downstream of the confining berm.



GRIT 3340

### Construction of the Trial Sand Berm

Figure 1 illustrates the proposed location for the trial cycloned sand berm ("Sand Berm") on the downstream face of the Perimeter Embankment, facing the Perimeter Embankment Seepage Collection Pond. The Sand Berm will be constructed in two phases, as illustrated in Figures 1 and 2. The existing access road running along the toe at Elevation 931m will be re-aligned for approximately 120 metres to allow vehicular traffic around the toe of the proposed berm. This will require approximately 900 cubic metres of Zone T material. The Sand Berm will be constructed above three outlet drains previously placed in the embankment in order to collect seepage water from chimney and longitudinal drains and direct it towards the seepage collection pond. These outlet drains will act as spine drains below the Sand Berm and their effect will be directly measurable by comparison of pore pressure monitoring results between instrumentation planes D and F. Type 2 geotextile filter fabric (8 oz./sq.yd.) will be extended over all outlet drain gravels in the area to be covered by the berm.

Figures 1 and 2 illustrate other major components of the two phases of Sand Berm construction:

### Confining Berms

A free-draining confining berm, 120 metres in length and 1.5 metres in height, consisting of Zone T Mine Rock, will be constructed to a crest elevation of approximately 932.5m along the existing access road to contain the ultimate toe of the berm. Two small confining berms on the embankment slope bench (Elev. 934), 0.5 metres in height, will also be placed to prevent travel of cycloned sand along the bench. The total volume of material needed for the confining berms is estimated to be 600 cubic metres.

### Pore Pressure Monitoring Instruments

Piezometers will be installed on two vertical planes cutting through the Sand Berm: one set of piezometers will be located on a plane coincident with Outlet Drain #4 (Section 1) and the other at a 20 metre distance from the nearest outlet drain (Section 2). RST vibrating-wire piezometers, Model VW2100L, which are capable of measuring pore pressures between -15 and +25 psi (-100 and +170 kPa), are recommended and will be read and recorded using a manual readout device. To

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minimize disturbance and provide good quality data, all piezometers will be installed prior to cyclone sand placement, on wood or steel posts driven into the existing embankment. These posts will be clearly marked at 0.5 metre elevation increments to allow for surveying of the cycloned sand and further interpretation of data.

### Water Management

Drainage ditches, excavated in low permeability till, will surround the Sand Berm in order to collect seepage and direct it to the Collection Pond. A culvert or suitable drain pipe will be installed across the existing access road. The existing drain monitoring sump will monitor flow through the outlet drains and a weir will be installed to measure flow through the seepage collection ditches. All tailings water will report to the Scepage Collection Pond for recycle to the tailings impoundment.

### Phase I Deposition of Cycloned Tailings

Once preparations are complete, deposition of approximately 2900 cubic metres of cycloned sand can be achieved through placement of one or two operating cyclones on the crest of the existing embankment. Figure 1 illustrates a plan view of and sections through the berm following Phase I of construction, assuming a 3.5:1 angle of repose for the cycloned sand.

Regular (minimum twice daily) monitoring of piezometric data and outlet drain flow will take place during Phase I deposition and will continue until full drainage of the berm has occurred. It is anticipated that drainage of the sand will continue for up to 10 days.

### Phase II Deposition of Cycloned Tailings

Deposition of approximately 3000 cubic metres of cycloned sand will then be undertaken to reach the configuration shown in the Figure 2 plan and sections. A 3.5:1 downstream slope and a 4:1 upstream slope allows for a 4m wide berm crest at Elevation 938m. Monitoring of piezometer, outlet drain and seepage flow will continue through Phase II and beyond until stabilized conditions are observed.

### Monitoring of the Trial Sand Berm

All 10 piezometers will be monitored regularly over the period from initial cycloning of tailings until pore pressure conditions stabilize in the trial berm. The outlet drain monitoring sump will record outlet drain flow and a weir will monitor flow through

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the drainage ditches constructed downstream of the trial berm. Sampling of cyclone output will determine underflow versus overflow split. An accurate determination of the material split for the cyclone will be done by directly measuring the concurrent flow rates and pulp densities from the underflow and overflow, through a timed discharge of the two streams.

Sampling and testing of outlet water quality will also be carried out at least once during Phase I and twice during Phase II. Elevation markers on the piezometer posts and a final survey will determine the typical angle of repose for cycloned sand material.

### Geotechnical Testing

A seismic Cone Penetration Testing program will be performed on tailings on the upstream side of the existing embankment and on cycloned sand on the downstream side. Triaxial shear tests are also to be performed on samples taken from the trial berm. Testing of permeability versus confining pressure and consolidation parameters will be carried out in addition to grain size distribution analyses. These tests will provide geotechnical data essential to the design of the TSF embankments using cycloned tailings.

#### Schedule for Construction and Monitoring

Construction would begin in early August, 1999 and be complete by the end of the same month. A tentative schedule is shown in Table 1. Based on the conservative assumptions of a 35% underflow split, 85% availability of the cyclone(s), a production rate of 20,000 tpd and a dry density of 1.6 tonnes/cubic metres, a single cyclone will deposit approximately 930 cubic metres of cycloned sand per day. Thus, with a single cyclone operating, Phase I deposition is expected to last 3 days, followed by a 10 day interruption to monitor drainage and 4 more days for completion of Phase II. The scenario of a single cyclone was used for the schedule in Table 1, however it may be possible to use two operating cyclones, which would decrease deposition time by 50 percent.

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#### Costs

Estimated costs for the construction materials, piezometer equipment, CPT and water quality testing programs are provided in Table 2.

### Summary

The trial study described above will evaluate techniques proposed to construct the TSF embankments using cycloned sand. Pore pressure, water management and geotechnical data from the trial berm will provide valuable insight into the behaviour of cycloned sand when used as embankment material when an impermeable core zone is retained. Concerns regarding stability of a cycloned sand embankment and the implications of tailings settlement on the sloping embankment core zone will be addressed with representative data collected during this trial study.

We trust the attached tables and figures provide the information you require. Please feel free to contact me should you have any questions or further requests.

Yours very truly,
KNIGHT PIÉSOLD LTD.

omginal signed by K.J. Brouwer

Ken J. Brouwer, P.Eng.

**Director** 

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Enclosure

MOUNT POLLEY MINING CORPORATION - MOUNT POLLEY MINE PROPOSED CONSTRUCTION AND MONITORING SCHEDULE

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[	S M T W T							•••• , , , ••••
22 August	S + 1   M   c				,			
15 August	M o							
08 August	1 1 1 M 1 1 M 1 1 M 1 1 M 1 M 1 M 1 M 1				· · · · · · · · · · · · · · · · · · ·		Addition	
Of August	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	<del></del>			<b></b>			
Pinich	02-08-99	04-08-99	04-08-99	04-08-99	07-08-99	66-80-16	21-08-99	24-08-99
Z. S.	66-80-10	03-08-99	03-08-99	04-08-99	05-08-99	05-08-99	18-08-99	22-08-99
Duration	2d	22	24	Id	ЭФ	27d	44	3d
Task Name	Access Road Re-Alignment	Construction of Confining Berms	Construction of Seepage Collection Ditches	Installation of Piezometers	Phase I Cycloning	Monitoring	Phase II Cycloning	Seismic CPT Program
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### TABLE 2

## MOUNT POLLEY MINING CORPORATION MOUNT POLLEY MINE

## CYCLONED-SAND BERM

MANII	<b>ペプい とういく</b> に	めんり	47 Y

Îtem		Quantity	Unit Rate		Cost
Construction Material				Ī	
Confining Berm (Zone ?	r)	600 m <sup>3</sup>	\$ 8.75 / m <sup>3</sup>	\$	5,250
Access Road Diversion	(Zone T)	900 m³	\$ 8.75 / m <sup>3</sup>	\$	7,875
Subtotal				\$	13,125
Piezometers					
Piezometer Number	Lead Length (m)				
D2-PE1-02	40				
D2-PE1-03	33.5				
D2-PE1-04	32.5				
D2-PE1-05	31.5				
D2-PE1-06	25.5				
F2-PE1-01	40				
F2-PE1-02	33.5	{			
F2-PE1-03	32.5				
F2-PE1-04	31.5				
F2-PE1-05	25.5				
Piezometers		10	\$ 830	s	8,300
Piezometer Cable		350 m	\$ 2.30 / m	\$	805
Subtotal			-	\$	9,105
CPT and Water Quality	y Testing				
Seismic CPT Program				\$	11,600
Water Quality Tests		3	\$ 750 ea.	\$	2,250
Subtotal				\$	13,850
Total Estimated Cost				\$	36,080

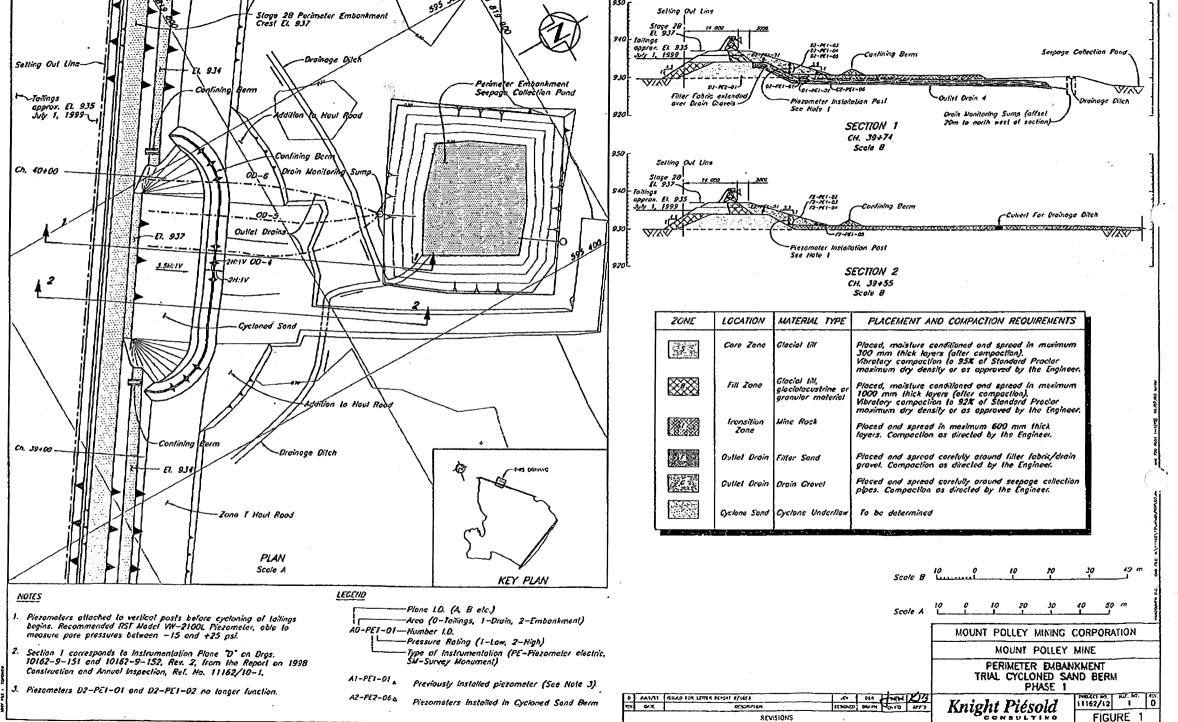
#### Notes:

- 1. Unit rate for load, haul, dump, spread and compact of Zone T material.
- 2. Piezometers are vibrating-wire, RST Model VW2100L rated from -15 to +25 psi, connected to existing readout panel at site via standard non-vented direct burial cable.
- 3. Estimate for Seismic CPT Program from Conetec Investigations Ltd.
- 4. Costs of geotechnical testing associated with Trial Sand Berm are included in budget for 11162/12, Task 300.

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