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MOUNT POLLEY MINING CORPORATION
MOUNT POLLEY MINE
TAILINGS STORAGE FACILITY

REPORT ON STAGE 4 CONSTRUCTION
(REF.NO. VA101-1/10-1)

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MOUNT POLLEY MINING CORPORATION
MOUNT POLLEY MINE
TAILINGS STORAGE FACILITY

REPORT ON STAGE 4 CONSTRUCTION
(REF.NO. VA101-1/10-1)

EXECUTIVE SUMMARY

The Mount Polley gold and copper mine is owned by Mount Polley Mining Corporation (MPMC). It is located 56 kilometres northeast of Williams Lake, in central British Columbia. Mount Polley Mine started production in 1997 and had milled approximately 27.5 million tonnes of ore prior to stopping production in October 2001. Mount Polley Mining Corporation commenced upgrading the mine facilities in the second half of 2004 and started production again in March 2005. MPMC has since been mining at an approximate rate of 18,000 tpd and received a permit on May 25, 2005 approving the Stage 4 construction of the Tailings Storage Facility, which involved raising the elevation of the Tailings Storage Facility embankments to an elevation of 948 m.

The Stage 4 construction program involved constructing an upstream cap on the Stage 3C embankment crests thereby raising the TSF embankments to an elevation of 948 m. The Stage 4 TSF construction program at Mount Polley Mine commenced in May 2005 and was completed in the first week of October 2006. Earthworks for the Stage 4 Tailings Storage Facility construction program comprised the following zones and materials:

- Zone S Fine grained glacial till.
- Zone U Upstream shell zone.
- Zone CBL Coarse Bearing Layer – rockfill.

Placement of Zone C material in the downstream Shell Zone commenced in April 2006. The shell zone construction is officially part of the Stage 5 construction and will be discussed in the Stage 5 construction report.

The results of the technical supervision and QA/QC testwork indicate that the fill materials placed and compacted on the tailings embankments were within the required material specifications and were in accordance with the Stage 4 design of the TSF.

A total of 22 of the functioning piezometers were accidentally damaged during Stage 4. MPMC and Knight Piésold attempted to locate and splice the damaged piezometers and successfully repaired five of them, leaving the total of functioning piezometers at 34. The results of the instrumentation monitoring show that no unexpected or anomalous pore pressures have developed. Additional piezometers will be installed during the Stage 5 construction program to compensate for those accidentally damaged during Stage 4. Details of the number and locations of the additional piezometers will be presented in the Stage 5 construction report.

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Three new inclinometers were installed downstream of the Main Embankment through the Lacustrine unit during Stage 4. This brings the total number of inclinometers to four at the Main Embankment, as inclinometer SI01-01 was damaged during the placement of shell zone material and is no longer functioning. The new inclinometers were read with an inclinometer probe to establish baseline data and a schedule for on-going monitoring was established. There have been no significant deviations in the two inclinometer casings installed in 2001.

The monitoring frequency of the vibrating wire piezometers and inclinometers following the Stage 4 construction program should be completed as outlined in the Operations and Maintenance Manual. The tailings pond elevation is monitored on a weekly basis to ensure that the stormwater and freeboard requirements are maintained during operations.

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SECTION 1.0 - INTRODUCTION

1.1 PROJECT DESCRIPTION

The Mount Polley gold and copper mine is owned by Mount Polley Mining Corporation (MPMC). It is located 56 kilometres northeast of Williams Lake, in central British Columbia. The project site is accessible by paved road from Williams Lake to Morehead Lake and then by gravel road for the final 12 km. Mount Polley Mine started production in 1997 and had milled approximately 27.5 million tonnes of ore prior to stopping production in October 2001. Mount Polley Mining Corporation commenced upgrading the mine facilities in the second half of 2004 and started production again in March 2005. MPMC has since been mining at an approximate rate of 18,000 tpd and received a permit on May 25, 2005 approving the Stage 4 construction of the Tailings Storage Facility, which involved raising the elevation of the Tailings Storage Facility embankments to an elevation of 948 m. An overall site plan of the Mount Polley Mine is shown on Drawing 100.

1.2 SCOPE OF REPORT

This report documents the Stage 4 construction program for the TSF. The report includes a discussion of the construction methods used to complete the work, the results of quality assurance tests, and review of the instrumentation monitoring results. The report also includes a set of “As -Built” drawings corresponding to the Stage 4 construction program.

SECTION 2.0 - STAGE 4 CONSTRUCTION PROGRAM

2.1 GENERAL

The Stage 4 TSF construction program at Mount Polley Mine commenced in May 2005 and was completed in the first week of October 2006. The construction program involved constructing a cap on the Stage 3C embankment crests thereby raising the TSF embankments to an elevation of 948 m. The construction of the tailings embankments has been an ongoing activity, and the Stage 4 construction program evolved into the Stage 5 construction program in October 2006 with a minimal break in the construction activities or construction supervision provided by Knight Piésold Ltd.

The general arrangement of the TSF is shown on Drawing 102. The material specifications are shown on Drawing 104. The Stage 4 Main Embankment Plan and Sections and Details are shown on Drawings 210 and 215 respectively. The Stage 4 Perimeter Embankment Plan and Section and Details are shown on Drawings 120 and 125 respectively. The Stage 4 South Embankment Plan and Section and Details are shown on Drawings 130 and 135 respectively. Select photographs of the construction program are included in Appendix D.

The main components of the TSF are as follows:

- The TSF embankments, which incorporate the following zones and materials:
  - Zone S Core zone - fine grained glacial till.
  - Zone CS Upstream shell - cycloned or spigotted tailings sand.
  - Zone B Embankment shell zones - fine grained glacial till.
  - Zone F Filter, drainage zones, and chimney drain - processed gravel and sand.
  - Zone T Transition filter zone - select well-graded fine-grained rockfill.
  - Zone C Downstream shell zone – rockfill.
  - Zone U Upstream shell zone – parameters vary depending on material availability.
  - Zone CBL Coarse Bearing Layer – rockfill.
- A low permeability basin liner (natural and constructed), which covers the base of the entire facility, at a nominal thickness of at least 2 m. The low permeability basin liner has proven to be effective in minimizing seepage from the TSF as there have been no indications of adverse water quality reporting to the groundwater monitoring wells (refer to Annual Reclamation Report for details).
- Embankment drainage provisions which include foundation drains, upstream toe drains, and chimney, longitudinal and outlet drains. The embankments drains have been incorporated into the design of the TSF to facilitate drainage of the tailings mass, dewater the foundation soils, and to control the phreatic surface within the embankments.
- Seepage collection ponds located downstream of the Main and Perimeter Embankments. These ponds were excavated in low permeability soils and store water collected from the embankment drains and from local runoff.
- Instrumentation in the tailings, earthfill embankments and embankment foundations. This includes vibrating wire piezometers, and slope inclinometers.

- A system of groundwater quality monitoring wells installed around the TSF.

The Stage 4 construction program involved raising the TSF embankments to an elevation of 948 m by constructing a 4.0 m cap on the Stage 3C crest elevation of 944 m. This involved placing Zone S and Zone U materials, and also included the placement of a coarse bearing layer on the tailings surface to create a suitable bearing surface to support the construction of the Zone U material. There was no placement of Zone F, Zone T, or Zone C materials during the Stage 4 construction program.

The Stage 4 program also involved installing three new inclinometers in the Lacustrine unit at the Main Embankment and the installation of piezometers in the tailings beaches beneath the coarse bearing layer.

Zone S material was also placed on the knoll between the South and Main Embankments to ensure that the basin liner in this area had a minimum thickness of 2.0 m.

2.2 QUALITY ASSURANCE/QUALITY CONTROL

Knight Piésold provided the Stage 4 design for the Tailings Embankments, prepared the Technical Specifications, provided technical assistance and performed quality assurance/quality control (QA/QC) testing during the construction Program. Key items addressed by Knight Piésold Ltd. included:

- Foundation inspection and approval prior to fill placement.
- Assessment of borrow material suitability.
- Inspection of fill placement procedures.
- In-situ testing of placed and compacted fill for moisture content and density.
- Collection and testing of Control and Record samples.
- Instrumentation monitoring.

Knight Piésold worked under the overall management and administration of MPMC. Lake Excavation and MPMC completed the construction work. The QA/QC procedures followed by Knight Piésold were similar to previous construction programs at the TSF. Material samples collected for laboratory testing during the construction program included Control and Record samples. The Control tests were carried out on materials collected from the borrow areas or from source locations to determine their suitability for use in the work. Record tests were performed on materials after placement and compaction to document the level of workmanship achieved and to ensure that the design objectives were met. The Control and Record test results are presented in Appendix A.

The Stage 4 construction program extended through the winter months of 2005/2006. The portion of the construction program that was completed during freezing conditions was monitored carefully by Knight Piésold to ensure that the work was carried out in accordance with the Technical Specifications.

2.3 STAGE 4 EARTHWORKS

2.3.1 General

Earthworks for the Stage 4 Tailings Storage Facility construction program comprised the following zones and materials:

- Zone S Fine grained glacial till.
- Zone U Upstream shell zone.
- Zone CBL Coarse Bearing Layer – rockfill.

The material specifications for the fill materials are shown on Drawing 104. The fill materials are discussed in the following sections.

2.3.2 Zone S

Zone S forms the low permeability core and seal zones for the Main, Perimeter and South Embankments. The material used in Zone S was fine grained glacial till from Borrow Area No. 2, which is located downstream of the left (East) abutment of the Main Embankment. The Control test results for the Zone S material are presented in Appendix A and summarized on Table 2.1. The results of the Control particle size analyses on the Zone S material are shown on Figure 2.1.

The Specification for Zone S material required placement and compaction in maximum 300 mm thick horizontal lifts. The compaction specification was 95 percent of the Standard Proctor maximum dry density. Each lift of Zone S was tested and approved prior to the placement of the subsequent lift. Areas that failed to meet the compaction requirements were re-compacted until the minimum compaction requirements were met. Material that did not meet the compaction requirements was typically too wet for use as construction material and was removed by pushing upstream of the crest onto the tailings beach.

Record tests on the compacted Zone S fill included the following:

- Moisture Content (ASTM D2216).
- Particle Size Distribution (ASTM D422).
- Laboratory Compaction (ASTM D698).
- Atterberg Limits (ASTM D4318).
- Field Density by Nuclear Methods (ASTM D2922).
- Field Moisture Content by Nuclear Methods (ASTM D3017).

A total of 23 Zone S Record samples were collected and tested in a soils laboratory during the Stage 4 construction program. A total of 15 of these samples were tested for atterberg limits, laboratory compaction, and moisture content, while all 23 of the record samples were tested for particle size distribution. The Record test results indicate that the well graded Zone S material is typically comprised of silty sand with some gravel and some clay. The Record test results for the Zone S material are presented in Appendix A and summarized on Table 2.2. The gradation curves of the Zone S Record Tests are shown on

Figure 2.2. The moisture content of the Record Samples ranged from 6.7 to 15.1 percent, with an average of 10.8 percent. The Standard Proctor Maximum Dry Density ranged from 1,950 to 2,100 kg/m^3, with an average of 2,032 kg/m^3. The plastic limits ranged from 13.7 to 19.1 percent, with an average of 16.2 percent. The liquid limits ranged from 21.6 to 29.1 percent, with an average of 24.7 percent. The plasticity index ranged from 5.6 to 11.4 percent, with an average of 8.6 percent. All of the Zone S Record test results were within the specified limits for the material. The results of the lab testing indicate that the Zone S material used for the Stage 4 construction program was consistent with the Zone S materials used in previous construction programs.

An additional 248 field density and moisture content tests were performed on the Zone S material using a nuclear densometer to assess the compacted density and moisture content. The compacted dry density ranged from 1,695 to 2,313 kg/m^3, with an average of 2,038 kg/m^3, with the compacted moisture content ranging from 6.5 to 20.0%, with an average of 10.9%. The percent compaction as compared to the Standard Proctor maximum dry density ranged from 83.5 to 106.9%, with an average of 99.7%. Compacted materials that failed to meet the compaction requirements were re-compacted until the minimum compaction requirements were met or the material was removed from the dam. The compacted dry density results are shown on Figure 2.3, with the percent compaction results shown on Figure 2.4. The compacted moisture content results are shown on Figure 2.5, with the deviation from the Standard Proctor optimum moisture content results shown on Figure 2.6. The nuclear densometer results are presented in Appendix C.

2.3.3 Zone U

Zone U forms the upstream shell zone immediately adjacent to Zone S and is required to provide upstream support of the Zone S material required for modified centerline construction. The material used for Zone U was random fill material from Borrow Area No. 3, which is located downstream of the left (East) abutment of the Main Embankment. Zone U was also constructed using sand cells along the Perimeter and South Embankments. The sand cells involved discharging tailings into constructed cells upstream of the embankment. The confining berms had culverts installed into them to allow for the water and fine materials to exit the cells and flow into the TSF. The coarse tailings sand that settled out into the cells was constantly worked with a dozer to ensure proper distribution within the cells, to compact the sand and to expedite the drainage of excess water through the culverts. This method of constructing Zone U proved to be effective if the required man-power was available. Attempts to construct the sand cells without a dozer working the material were not successful and the resulting material was not approved by the Engineer. This unapproved material was pushed into the TSF with a dozer and the sand cell process was restarted. Sand cells were constructed on the Perimeter Embankment as well as on the South Embankment between Ch. 6+50 and 9+75.

Lab testing was performed on 11 Zone U record samples to determine particle size distributions (ASTM D422). The Record Tests indicate that the Zone U material from Borrow Area No. 3 generally consisted of gravelly sand, with the fines content ranging from 3 to 61%. The Zone U gradations from the sand cells indicate that this material generally consisted of fine sand. The gradation curves of the Zone U Record Tests are shown on Figure 2.7. The Photographs showing the construction of the sand cells are included in Appendix D.

2.3.4 Coarse Bearing Layer

A Coarse Bearing Layer (CBL) was placed on top of the tailings beach adjacent to the embankments to provide a suitable bearing surface for the Zone U material. The material consisted of waste rock and was placed using 777 haul trucks. The speed of the fill placement was carefully monitored during the placement of the CBL to ensure that the tailings below the CBL did not liquefy.

2.4 INSTRUMENTATION MONITORING

2.4.1 Vibrating Wire Piezometers

A total of 57 vibrating wire piezometers have been installed at the TSF along eight planes designated as Monitoring Plans A to H. The monitoring planes for the Main Embankment, the Perimeter Embankment, and the South embankment are shown on Drawings 250, 252, and 254 respectively. A summary of the instrumentation installations and typical details is shown on Drawing 256. The piezometer locations for the monitoring planes are shown in section on Drawings 258, and 259. The piezometers are grouped into tailings, foundation, embankment fill and drain piezometers. The piezometers were discussed in detail in the Knight Piésold Ltd. “Report on 2005 Annual Inspection, (Ref. No. VA101-01/11-1).

Thirteen months of piezometer data is missing from July 30, 2003 to September 2, 2004, and no piezometer data was collected from Sept 22, 2005 to April 30 2006. The current gap in missing piezometer data was due to a malfunctioning readout box connecter cable and the accidental destruction or burying of piezometer cables during the Stage 4 construction program.

There were 51 piezometers still functioning at the start of the Stage 4 construction program. A total of 22 piezometers were accidentally destroyed during the Stage 4 construction program. MPMC and Knight Piésold attempted to locate and splice the damaged piezometers and successfully repaired five of them. The piezometer readings were resumed for the piezometers that were damaged once the cables were repaired and the timeline plots updated. The pieziometers that were not damaged during the construction program were read on a weekly basis. The number of functioning piezometers has now been reduced to 34. Additional piezometers will be installed during the Stage 5 construction program to compensate for those accidentally damaged during Stage 4.

No unexpected or anomalous pore pressures were observed while monitoring the vibrating wire piezometers during the construction program. The timeline plots for the piezometers on planes A through H are shown on Figures 2.8 to 2.15 respectively. The timeline plots indicate that the pore pressures increased slightly in piezometers A2-PE2-03, B2-PE2-03, and B2-PE1-02, which are fill piezometers installed in the Zone S glacial till. These pore pressure increases were expected as these piezometers have shown similar trends in previous construction programs where the pore pressures have increased during fill placement activities and subsequently decreased following the construction programs as the pore pressures dissipate.

2.4.2 Slope Inclinometers

A total of three new slope inclinometers were installed downstream of the toe of the Main Embankment during the Stage 4 construction program. One of the inclinometers installed in 2001 (SI01-01) was damaged during the placement of the shell zone material and is no longer functioning. The last reading for SI01-01 was March 2006. There are four functioning inclinometers installed at the Main Embankment. The drill logs, installation details, and lab results for the three new inclinometers are included in Appendix B.

A 'poor-boy' monitoring rod was also used twice a month during the construction program to ensure that casing deformation due to soil movement associated with settlement or instability could be identified. MPMC purchased an inclinometer probe in August 2006 and the slope inclinometers are now being read once per month with the new probe to monitor any movement in the Main Embankment and the underlying lacustrine unit.

The results of the inclinometer readings and 'poor-boy' measurements indicate that there have not been any significant deviations measured in the inclinometers since their installation. There were no measurable impacts on the inclinometers resulting from the Stage 4 construction program. The results of the readings for inclinometers SI01 to SI05 are shown on Figures 2.16 to 2.20 respectively.

2.4.3 Drain Flow Data

The upstream toe drain and foundation drains at the Main Embankment flow into the sump at the Main Embankment Seepage Collection Pond where the flows are measured. The flow rates have been measured since July 2000; however the flow rates from the drains were not monitored during the Care and Maintenance Period as the drain outlets were submerged within the sump. This condition was anticipated as flow monitoring is only possible during operations when the seepage pond level has been pumped down. The seepage pond was pumped down in December 2005 and flow measurements were taken. The flow rates for the Main Embankment upstream toe drain are shown on Figure 2.1, which illustrates that the flows have increased since 2005, with the current flows ranging from 9 to over 13 l/s. The flow rates for the Main Embankment foundation drains are shown on Figure 2.22, which shows that flows have remained fairly constant

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since the flow measurements resumed in December 2005, and range from near 0 zero to about 1.8 l/s.

Samples from the Foundation Drains and the Upstream Toe Drain are collected by MPMC for water quality testing. The results are available from MPMC and are reported in the Annual Environmental Reports.

A new foundation drain was added at the South and Main Embankment junction between chainages 14+00 and 16+00 to intercept seepage in underlying fractured bedrock in this area and route it to the Main Embankment Seepage Collection and Recycle Pond.

2.4.4 Survey Monuments

Six survey monuments were installed on the Stage 3B embankment crest following the 2001 construction. These have since been covered during subsequent construction programs. The initial plan was to install additional survey monuments on the embankment crests following the completion of the Stage 4 construction program; however, this was not practical due to the ongoing construction of the TSF embankments. Monuments will be established in the summer of 2006.

2.5 DESIGN MODIFICATIONS

Knight Piésold Ltd. employs a strict procedure for making design modifications (changes or substitutions) in the field. All design change requests are submitted in writing by the Resident Engineer to the Knight Piésold Ltd. Vancouver Office for review and evaluation. If approved by the Design Engineer and Project Principal, the design change request is forwarded to the Owner and Contractor in a formal, written decision.

The design modifications implemented during the Stage 4 construction program were as follows:

- The fine limit of the Zone U material was adjusted to allow for the use of the coarse tailings sand as a construction material.
- A foundation drain was added at the approximate chainages of 14+00 and 16+00 to intercept seepage encountered at this area. The flows were routed to the Main Embankment Seepage Collection Pond.

SECTION 3.0 - SUMMARY AND RECOMMENDATIONS

Stage 4 of the Mount Polley Mine Tailings Storage Facility was constructed between May 2005 and October 2006. The Stage 4 construction program involved raising the TSF embankments to an elevation of 948 m, which involved placing a 4 m cap on the existing Stage 3C crest of 944 m. This involved placing Zones S and Zone U materials within an upstream raise that extended partially on top of the sandy tailings beaches.

Coarse tailings sand was used as Zone U material in places by developing sand cells and discharging tailings directly into the cells. This proved to be an effective way of constructing Zone U but required a full time dozer to segregate the full tailings stream. otherwise the material had to be wasted into the TSF as it did not drain properly.

Low permeability glacial till or “Zone S material” was also placed on the knoll between the South and Main Embankments to ensure that the basin liner in this area had a minimum thickness of 2.0 m.

The results of the Stage 4 technical supervision and QA/QC testwork indicate that the fill materials placed and compacted on the tailings embankments were within the required material specifications and were in accordance with the Stage 4 design of the TSF.

Three new inclinometers were installed at the Main Embankment downstream of the ultimate toe to provide a means of measuring potential deflections in the Lacustrine unit. Inclinometer SI01-01, which was installed in 2001 was damaged during placement o the shell zone material and is no longer functioning. The total number of inclinometers at the Main Embankment is now four. There have been no significant deflections measured in any of the inclinometers.

Technical supervision of the work by Knight Piésold included QA/QC testing and monitoring the existing vibrating wire piezometers and inclinometers. The QA/QC testing included collecting and testing Record samples, and testing the compacted fill materials using a nuclear densometer. The results of the QA/QC testwork indicate that the fill materials placed and compacted on the tailings embankments were within the required material specifications and were in accordance with the Stage 4 design of the TSF.

The piezometers were measured on a weekly basis using a VWP Indicator readout box and the inclinometers were measured twice a month using a “poor boy” probe. The inclinometers were also read using a SINCO inclinometer probe to provide a more detailed assessment of any significant deviations in the inclinometer casing since their installation in 2001. The results of the instrumentation monitoring show that no unexpected or anomalous pore pressures were observed while monitoring the vibrating wire piezometers and there were no measurable impacts on the inclinometers during the construction program. MPMC has purchased an inclinometer probe and measurements are now completed on a monthly basis.

The vibrating wire piezometers, inclinometers, and survey monuments should be read continually throughout the year as outlined in the Operations and Maintenance Manual.

The TSF is required to have sufficient live storage capacity for containment of runoff from the 24-hour PMP, in addition to regular inflows from other precipitation runoff, including the spring freshet, while maintaining the minimum freeboard requirements. The tailings pond elevation should be monitored on a regular basis to ensure that the stormwater and freeboard requirements are maintained during operations.

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SECTION 4.0 - CERTIFICATION

This report was prepared and approved by the undersigned.

Prepared by: March 15/2007

Les Galbraith, P.Eng.
Senior Engineer

Approved by: Mar 15, 2007

Ken J. Brouwer, P.Eng.
Managing Director

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TABLE 2.1

MOUNT POLLEY MINING CORPORATION
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STAGE 4 CONSTRUCTION PROGRAM

ZONE S CONTROL SAMPLES - SUMMARY

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Revised: 05-Mar-07

Sample	Atterberg Limits			MC	Grain Size Analysis				Standard Proctor				MC
No.					Gravel	Sand	Silt	Clay	Uncorrected		Corrected		Deviation From Optimum (%)
	L.L. (%)	P.L. (%)	P.I. (%)	M.C. (%)	> #4 (%)	#4 to #200 (%)	#200 to .002 (%)	< .002 (%)	Max D.D. (kg/m³)	Opt. M.C. (%)	Max D.D. (kg/m³)	Opt. M.C. (%)	
KP06-ZS-04C	18.9	18.0	2.9	14.2	10	19	58	13	1980	11.5	2030	10.5	3.7
KP06-ZS-05C	23.5	14.2	9.3	11.2	20	32	35	13	2040	10.5	2140	8.5	2.7
KP06-ZS-06C	23.3	14.2	9.1	10.4	18	30	39	13	2020	10.5	2090	9.5	0.9
KP06-01-C	25.0	15.7	9.3	13.2	16	40	27	17	2012	11.3	2092	9.7	3.5
KP06-02-C	31.9	20.0	11.9	15.8	18	34	31	18	1970	12.5	2059	10.6	5.2
KP05-88	25.2	16.9	8.3	11.3	8	34	58		2040	12.0	2085	11.0	0.3
KP05-93	23.4	14.6	8.8	7.6	19	34	47		2030	11.0	2131	9.1	-1.5
KP05-79	*N/A*	*N/A*	*N/A*	*N/A*	6	36	58		1900	15.5	1930	14.7	*N/A*
KP05-74	*N/A*	*N/A*	*N/A*	*N/A*	16	38	46		1990	12.5	2068	10.8	*N/A*
KP05-60	25.1	18.6	6.5	12.9	18	34	48		2080	10.5	2162	8.8	4.1
KP05-61	23.3	15.7	7.6	10.9	20	34	46		2080	10.5	2174	8.6	2.3
KP05-58	*N/A*	*N/A*	*N/A*	*N/A*	13	36	51		1970	13.0	2039	11.4	*N/A*
AVERAGE	24.4	16.4	8.2	11.9	15	33	42	15	2009	12	2083	10.3	2.4
MAXIMUM	31.9	20.0	11.9	15.8	20	40	58	18	2080	15.5	2174	14.7	5.2
MINIMUM	18.9	14.2	2.9	7.6	6	19	27	13	1900	10.5	1930	8.5	-1.5

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TABLE 2.2

MOUNT POLLEY MINING CORPORATION
MOUNT POLLEY MINE
STAGE 4 CONSTRUCTION PROGRAM

ZONE S RECORD SAMPLES - SUMMARY

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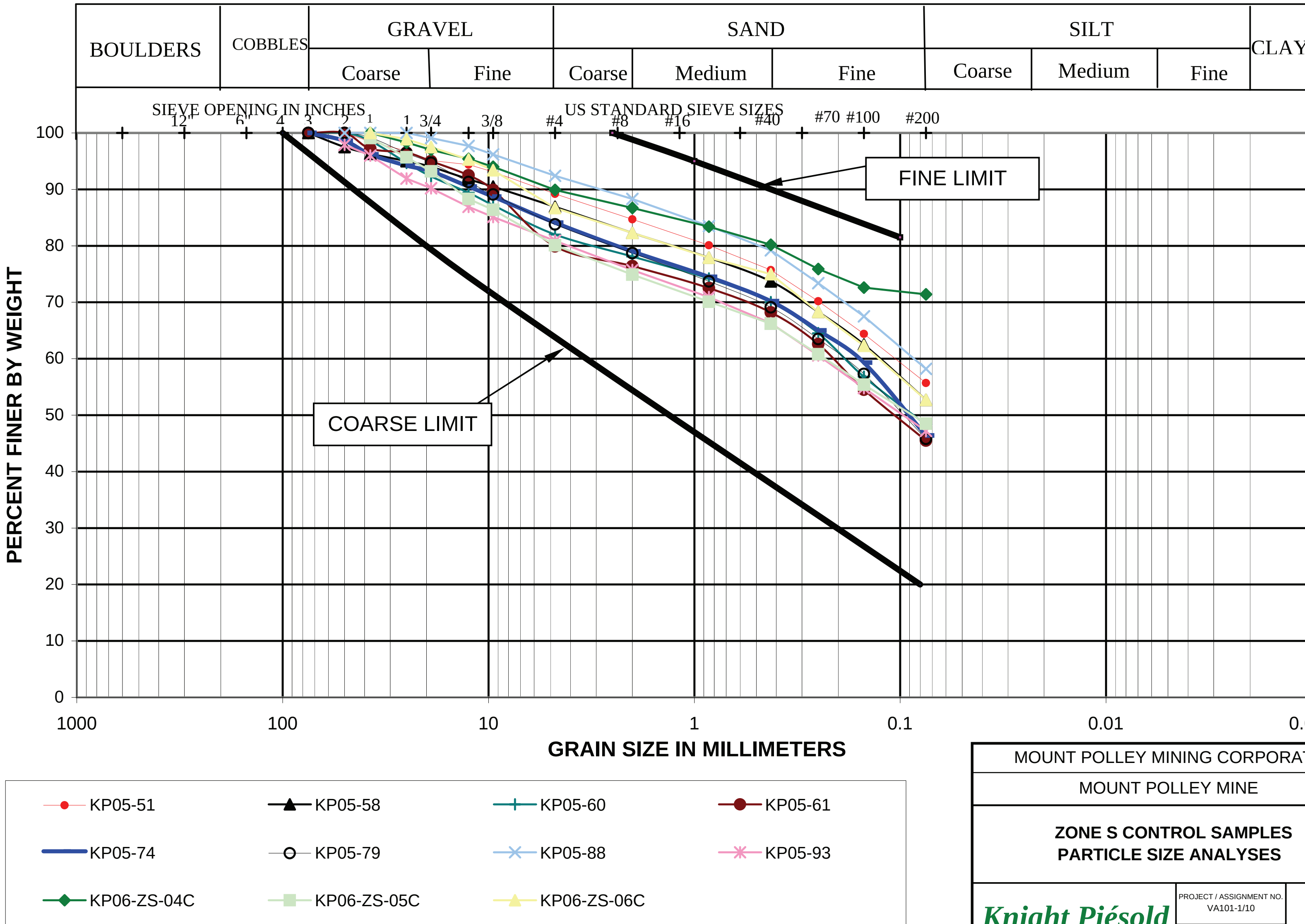
Revised: 05-Mar-07

Sample No.	Atterberg Limits			MC	Grain Size Analysis				Standard Proctor				MC
					Gravel	Sand	Silt	Clay	Uncorrected		Corrected		Deviation From Optimum (%)
	L.L. (%)	P.L. (%)	P.I. (%)	M.C. (%)	> #4 (%)	#4 to #200 (%)	#200 to .002 (%)	< .002 (%)	Max D.D. (kg/m³)	Opt. M.C. (%)	Max D.D. (kg/m³)	Opt. M.C. (%)	
KP-05-57	*N/A*	*N/A*	*N/A*	11.3	24	34	42		2100	10.5	2211	8.2	3.1
KP05-59	*N/A*	*N/A*	*N/A*	14.1	5	20	75		1960	12.0	1984	11.5	2.6
KP-05-62	23.6	15.6	8.0	12.3	22	30	48		2040	11.0	2145	8.9	3.4
KP05-63	26.3	14.9	11.4	11.4	18	30	52		2050	11.0	2133	9.3	2.1
KP05-64	24.3	13.7	10.6	11.9	14	34	52		2090	9.5	2153	8.3	3.6
KP05-65	25.9	15.9	10.6	10.7	18	32	50		2060	11.5	2142	9.7	1.0
KP05-66	22.0	15.8	6.2	10.4	20	32	48		2050	11.5	2139	9.6	0.8
KP05-67	25.7	17.9	7.8	10.3	15	33	52		2070	10.5	2141	9.1	1.2
KP05-68	21.6	16.0	5.6	9.6	19	34	47		2050	11.0	2140	9.1	0.5
KP-05-76	*N/A*	*N/A*	*N/A*	*N/A*	12	38	50		2010	12.0	2066	10.8	*N/A*
KP-05-77	*N/A*	*N/A*	*N/A*	*N/A*	12	39	49		2000	11.5	2056	10.3	*N/A*
KP-05-78	*N/A*	*N/A*	*N/A*	*N/A*	16	38	46		2040	11.0	N/A	N/A	*N/A*
KP05-80	*N/A*	*N/A*	*N/A*	*N/A*	12	36	52		2010	12	2069	10.7	*N/A*
KP-05-83	*N/A*	*N/A*	*N/A*	*N/A*	13	33	54		1990	11.5	2055	10.2	*N/A*
KP-05-84	*N/A*	*N/A*	*N/A*	*N/A*	10	32	58		1970	13.0	2024	11.8	*N/A*
KP05-85	26.8	17.3	9.5	11.1	11	32	57		2000	13.0	2054	11.7	-0.6
KP05-86	23.8	15.4	8.4	6.7	12	38	50		2060	10.5	2114	9.4	-2.7
KP05-87	26.6	17.8	8.8	8	10	33	57		2020	11.5	2070	10.4	-2.4
KP05-91	25.4	15.7	9.7	10.9	10	35	55		2010	12.5	2059	11.4	-0.5
KP05-92	23	15.7	7.4	8.7	24	31	45		2040	11.5	2078	10.7	-2.0
KP05-94	23.4	17.3	6.1	11.1	22	34	44		2080	9.5	2186	7.7	3.4
KP05-95	29.1	19.1	10	15.1	19	37	44		1950	13.0	2052	10.8	4.3
KP06-ZS-01R	23.7	14.3	9.4	10.1	19	33.9	47		2080	9.5	2170	8.0	2.1
AVERAGE	24.7	16.2	8.6	10.8	16	33	51		2032	11.3	2102	9.9	1.2
MAXIMUM	29.1	19.1	11.4	15.1	24	39	75		2100	13.0	2211	11.8	4.3
MINIMUM	21.6	13.7	5.6	6.7	5	20	42		1950	9.5	1984	7.7	-2.7



MOUNT POLLEY MINING CORPORATION
MOUNT POLLEY MINE
ZONE S CONTROL SAMPLES **PARTICLE SIZE ANALYSES**

Knight Piésold CONSULTING

PROJECT / ASSIGNMENT NO.	REF NO.
VA101-1/10	1
FIGURE 2.1	REV. 0

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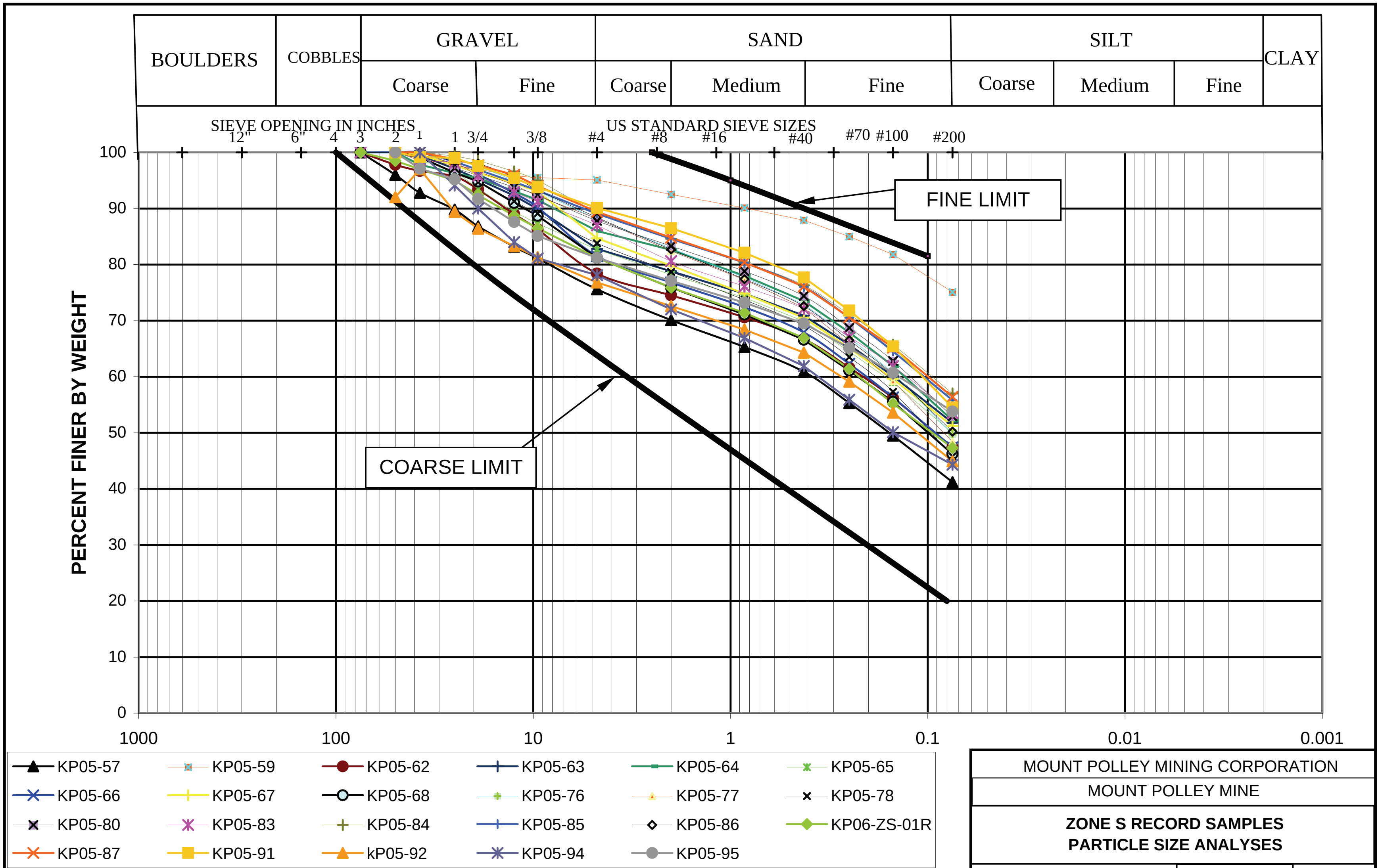

MOUNT POLLEY MINING CORPORATION

MOUNT POLLEY MINE

ZONE S RECORD SAMPLES
PARTICLE SIZE ANALYSES

Knight Piésold CONSULTING

PROJECT / ASSIGNMENT NO.	REF NO.	REV.
VA101-1/10	1	0

FIGURE 2.2

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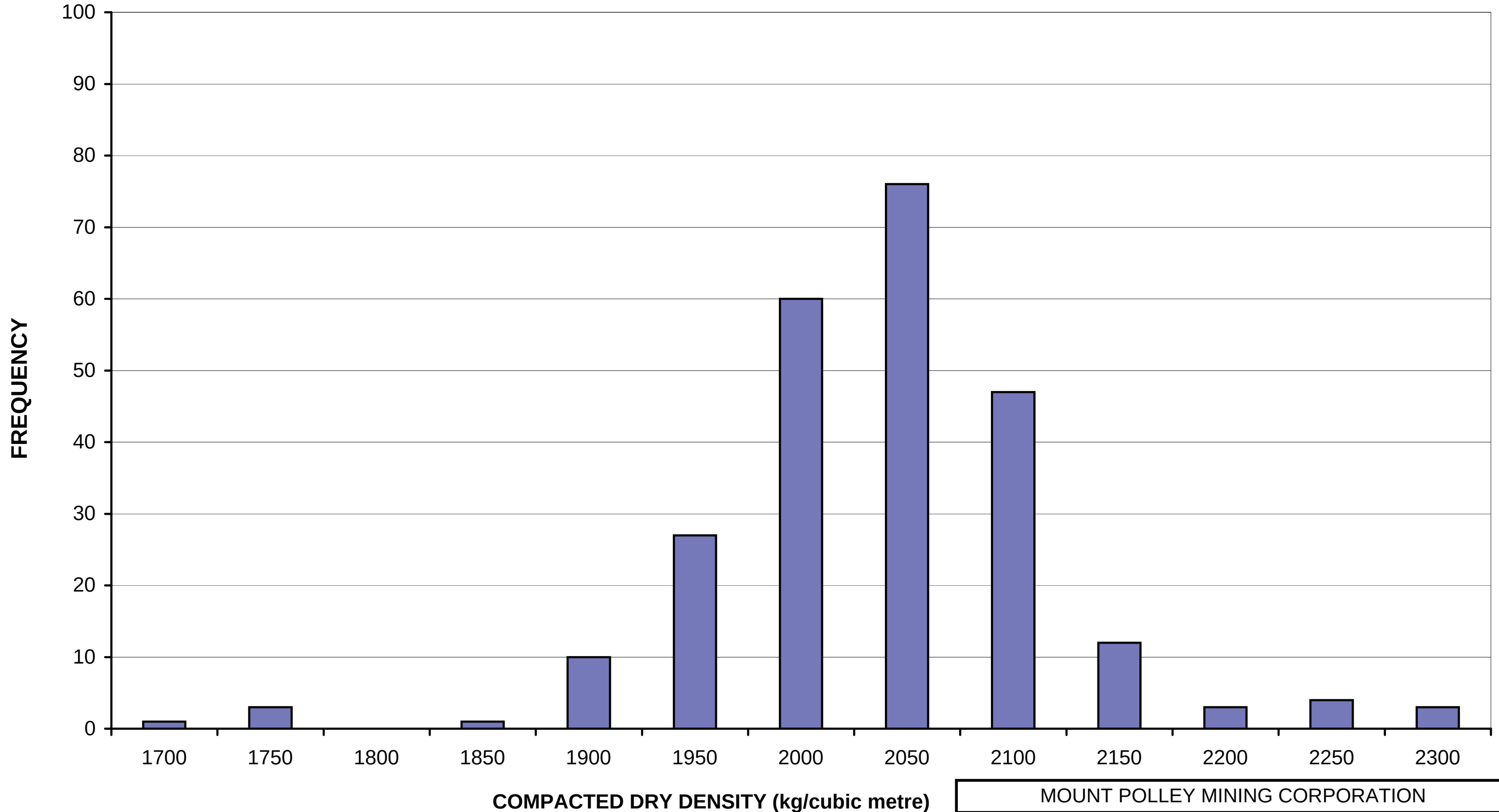

Notes:
1. The compacted dry density was measured using a nuclear densometer.

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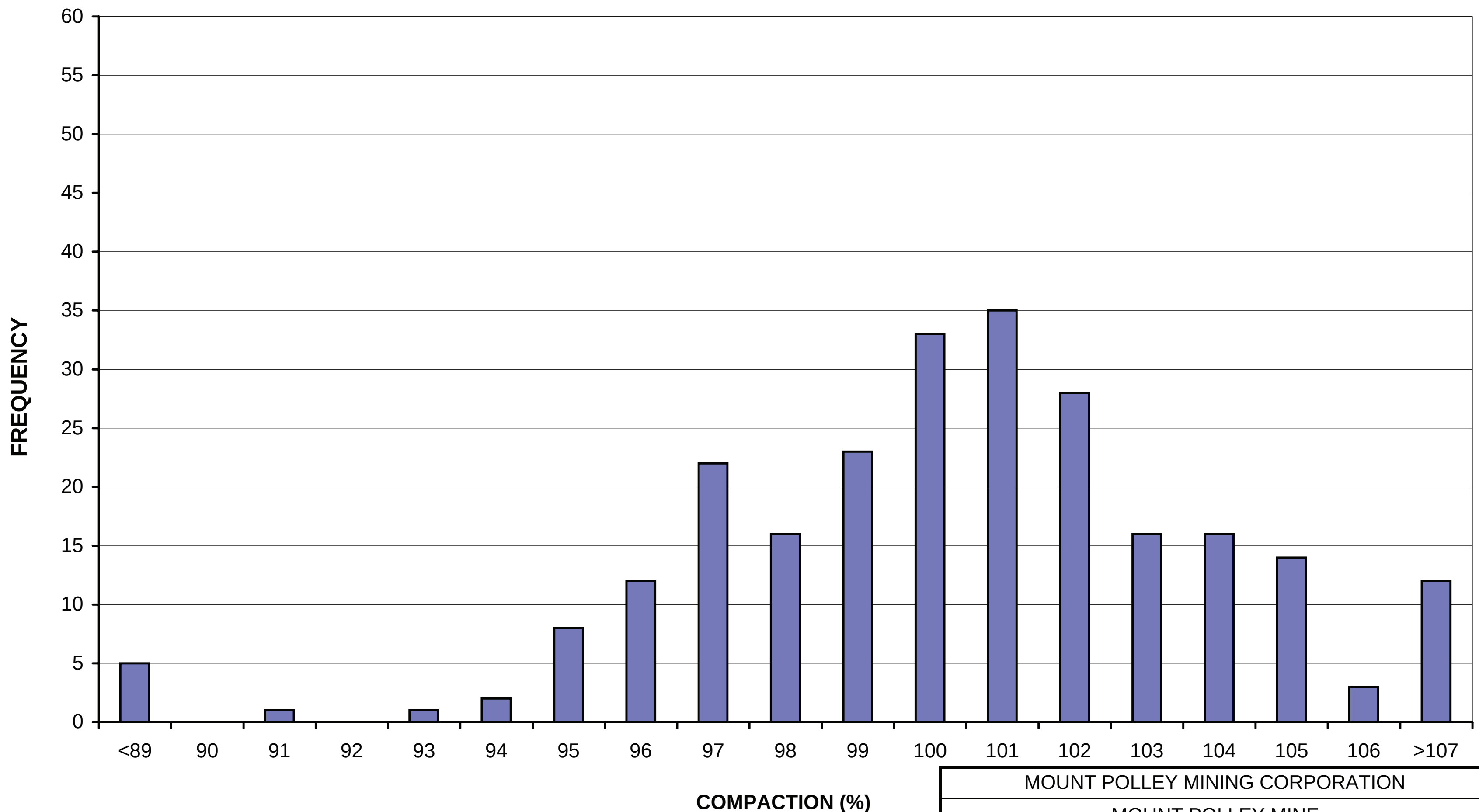
MOUNT POLLEY MINING CORPORATION		
MOUNT POLLEY MINE		
ZONE S RECORD TESTS **DRY DENSITY**		
Knight Piésold CONSULTING	PROJECT / ASSIGNMENT NO. VA101-1/10	REF NO. 1
	FIGURE 2.3	REV. 0



Notes:
1. The field dry density was measured using a nuclear densometer. The nuclear densometer test results compared to Zone S control and record tests for comparison.

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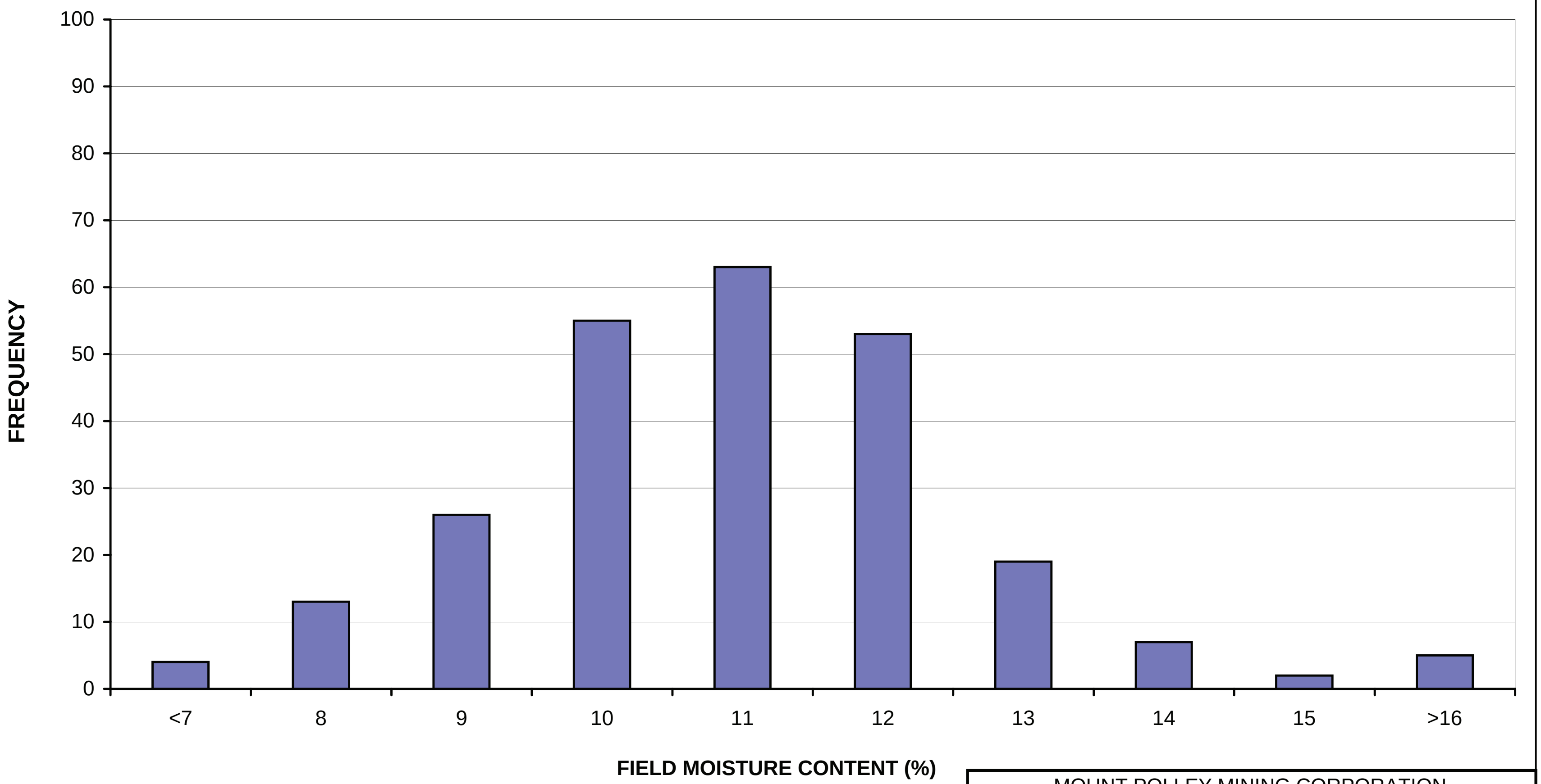
MOUNT POLLEY MINING CORPORATION	
MOUNT POLLEY MINE	
ZONE S RECORD TESTS **PERCENT COMPACTION**	
Knight Piésold CONSULTING	PROJECT / ASSIGNMENT NO. VA101-1/10; REF NO. 1
	FIGURE 2.4; REV. 0



Notes:
1. The compacted moisture content was measured using a nuclear densometer.

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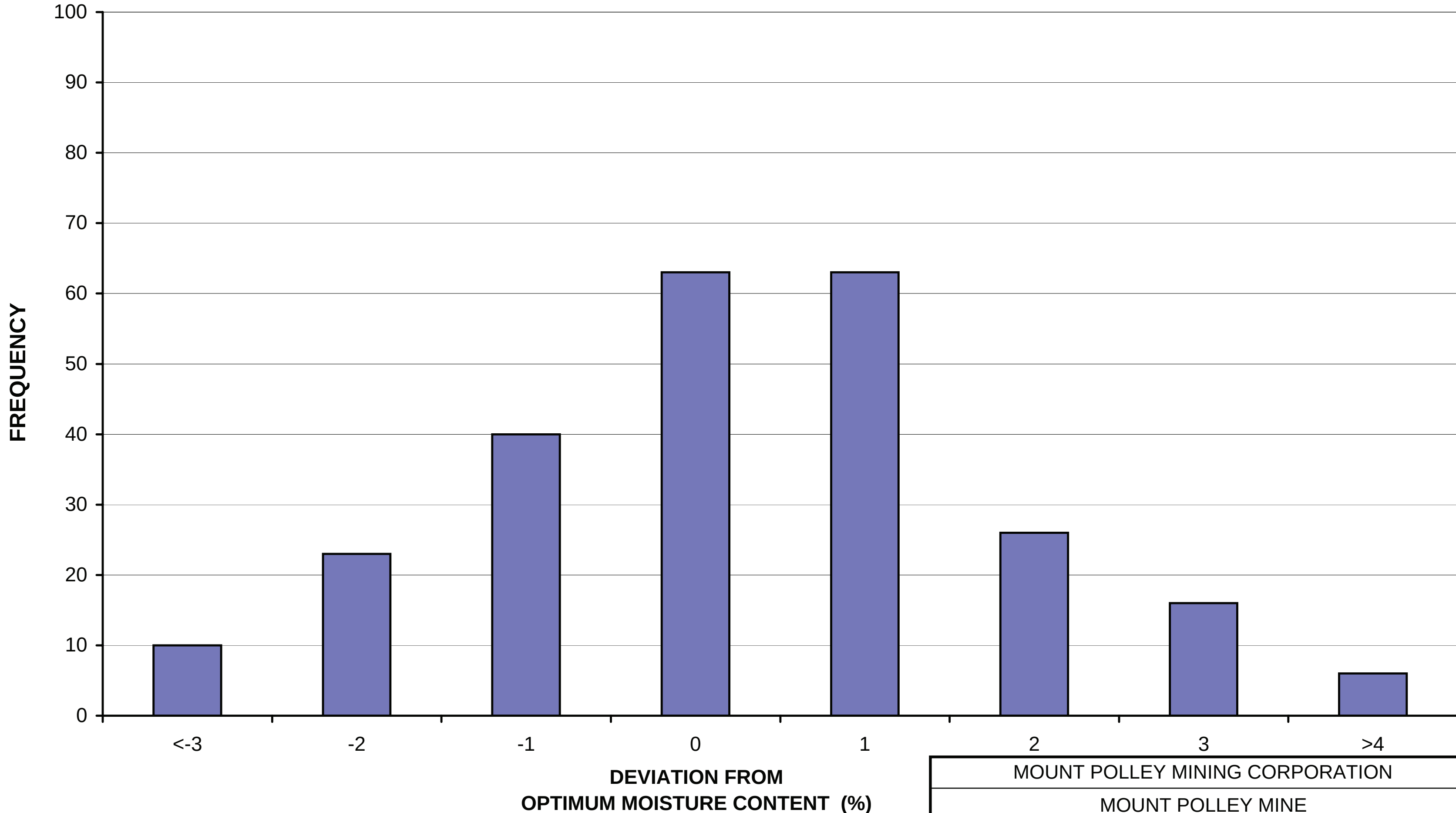
MOUNT POLLEY MINING CORPORATION

MOUNT POLLEY MINE

ZONE S RECORD TESTS
MOISTURE CONTENT

Knight Piésold CONSULTING

PROJECT / ASSIGNMENT NO.	REF NO.	
VA101-1/10	1	
FIGURE 2.5		REV. 0



Notes:
1. The Optimum Moisture Content refers to the Standard Proctor Optimum Moisture Content.
2. The compacted moisture content was measured using a nuclear densometer. The nuclear densometer test results compared to Standard Proctor Optimum Moisture Content for comparison.

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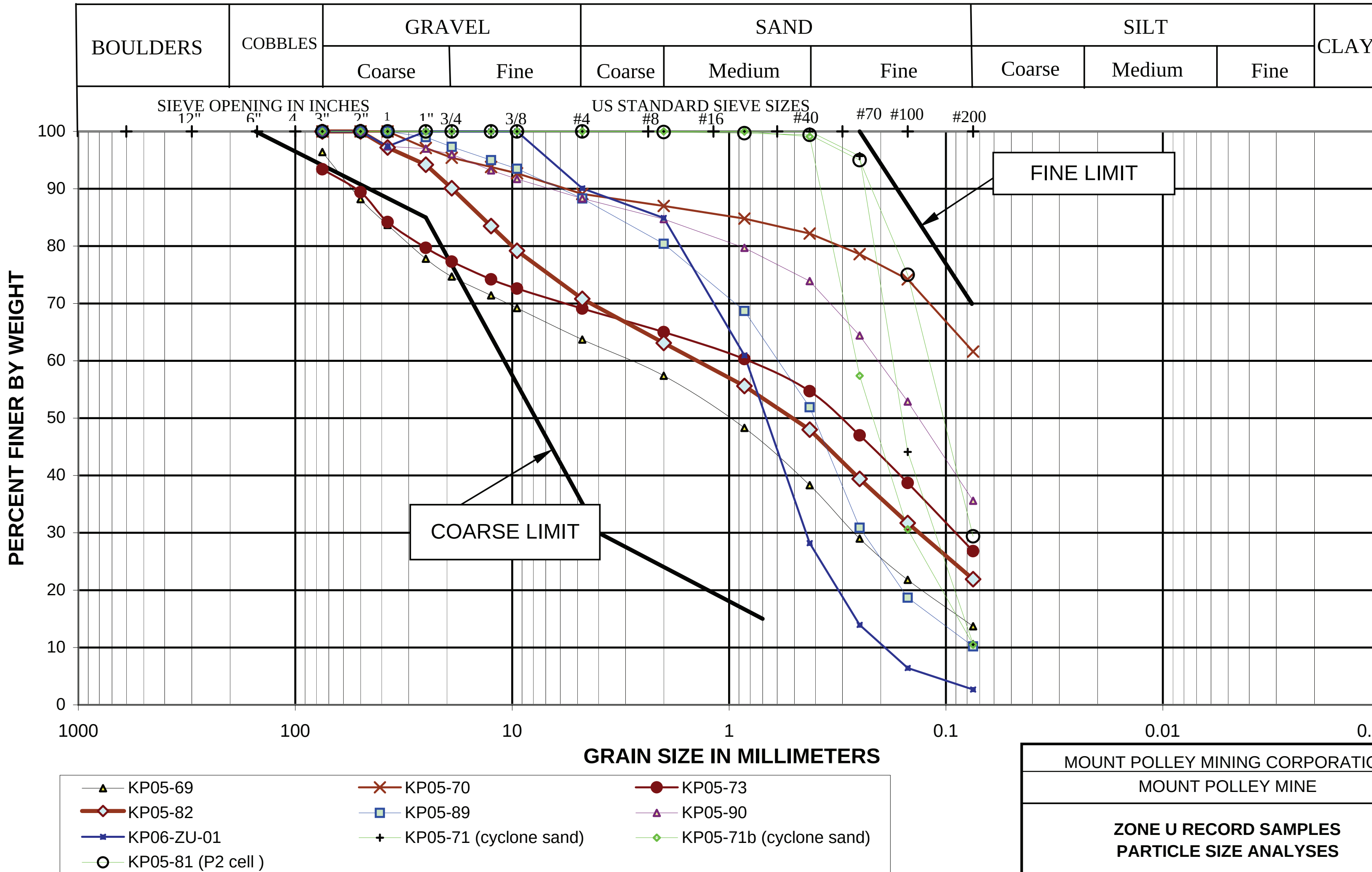
MOUNT POLLEY MINING CORPORATION		
MOUNT POLLEY MINE		
ZONE S RECORD TESTS DEVIATION FROM OPTIMUM MOISTURE CONTENT		
Knight Piésold CONSULTING	PROJECT / ASSIGNMENT NO. VA101-1/10	REF NO. 1
	FIGURE 2.6	REV. 0



MOUNT POLLEY MINING CORPORATION

MOUNT POLLEY MINE

ZONE U RECORD SAMPLES
PARTICLE SIZE ANALYSES

Knight Piésold CONSULTING	PROJECT / ASSIGNMENT NO. VA101-1/10	REF NO. 1
	FIGURE 2.7	REV. 0

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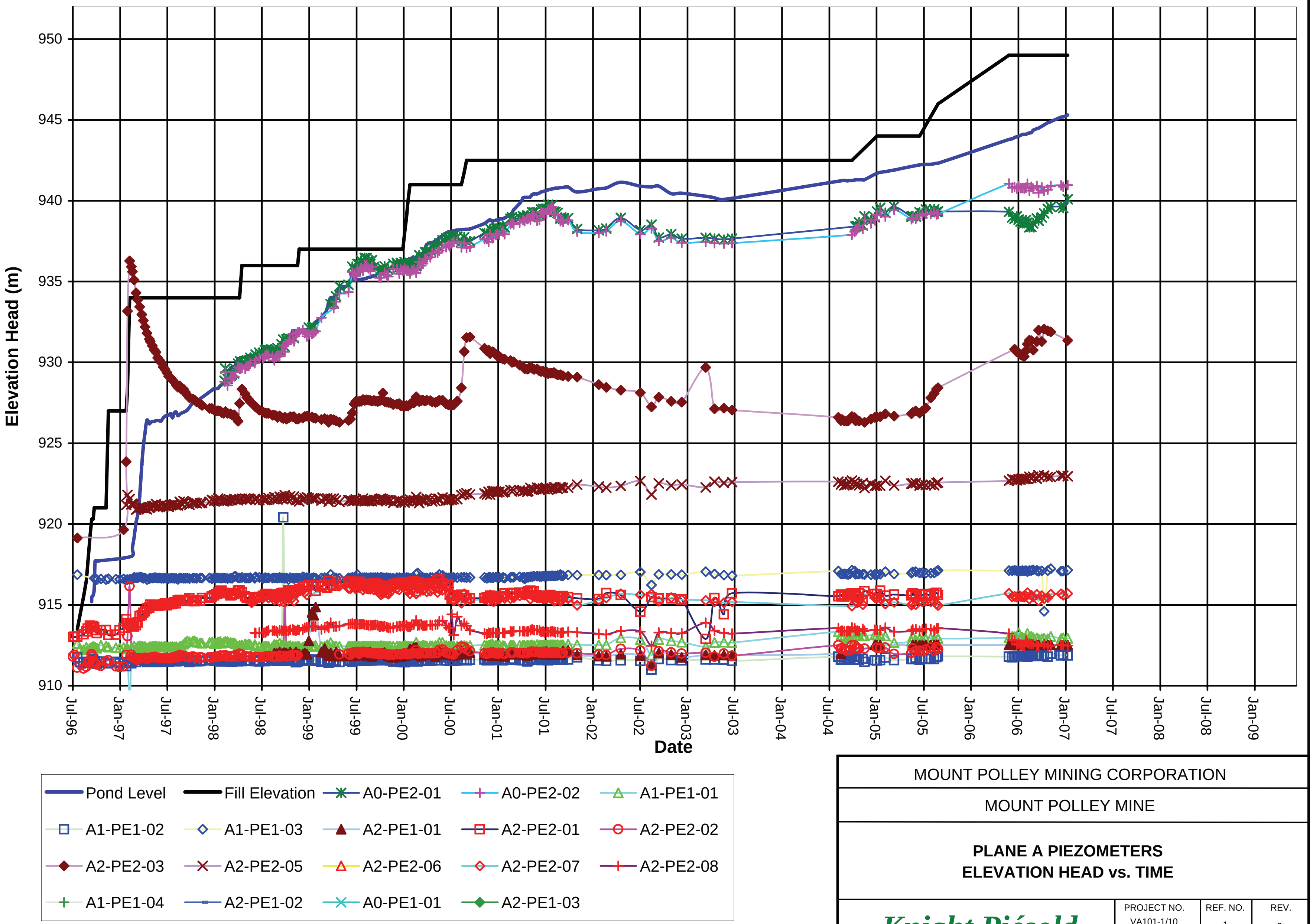

MOUNT POLLEY MINING CORPORATION		
MOUNT POLLEY MINE		
PLANE A PIEZOMETERS ELEVATION HEAD vs. TIME		
PROJECT NO. VA101-1/10	REF. NO. 1	REV. 0
FIGURE 2.8		

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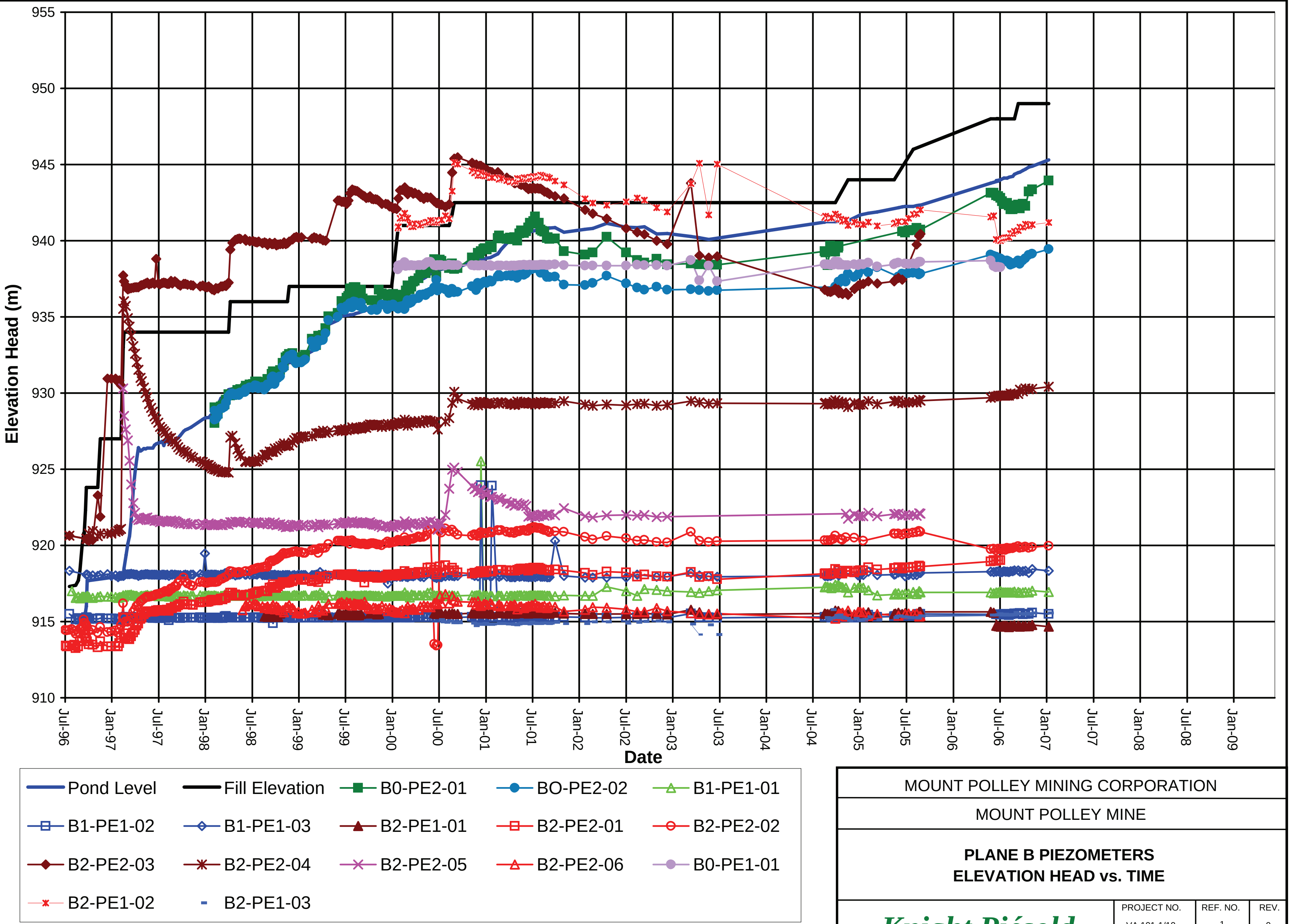

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MOUNT POLLEY MINING CORPORATION

MOUNT POLLEY MINE

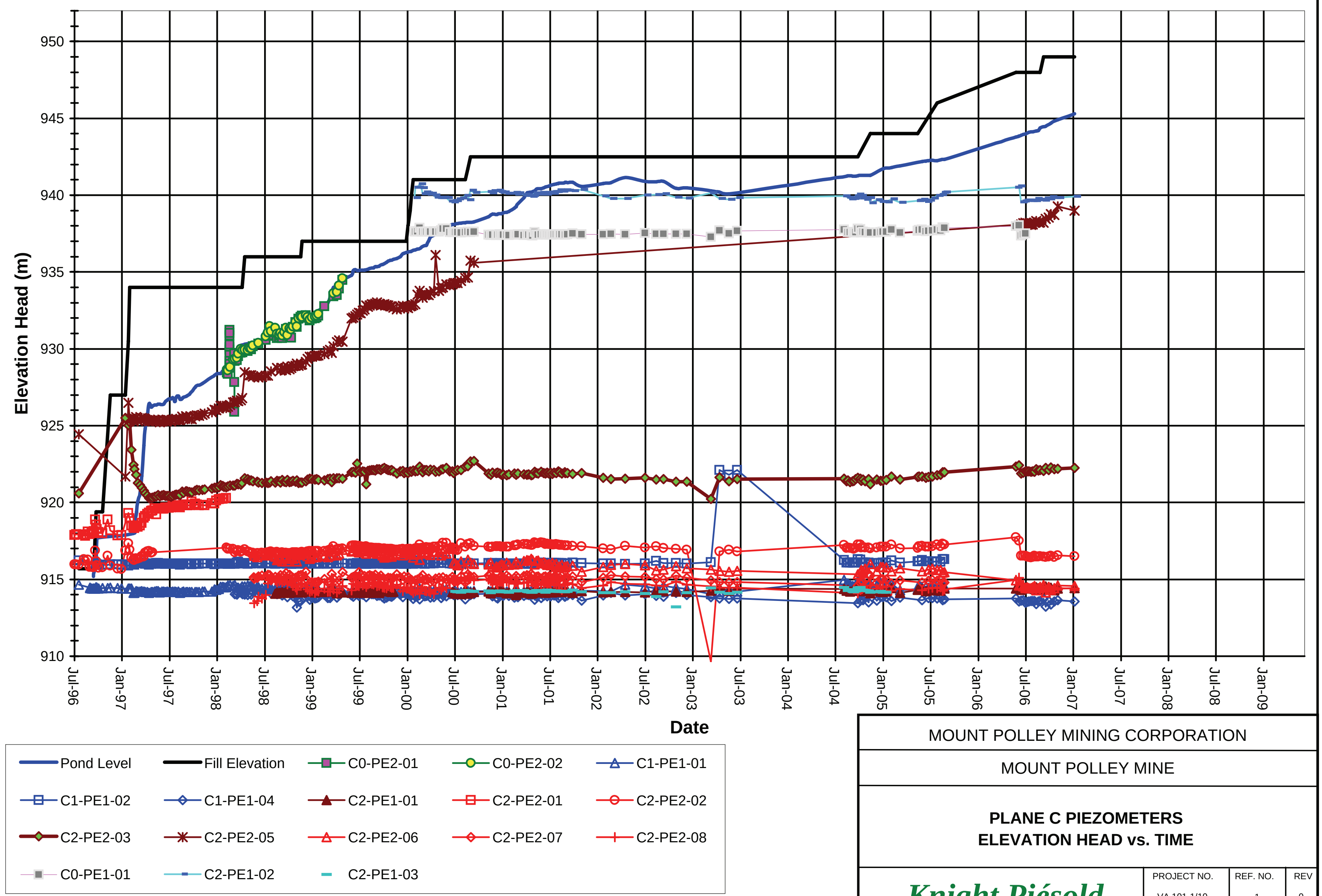
PLANE B PIEZOMETERS
ELEVATION HEAD vs. TIME

Knight Piésold CONSULTING

PROJECT NO.	REF. NO.	REV.
VA 101-1/10	1	0

FIGURE 2.9

AMEC010469_0028



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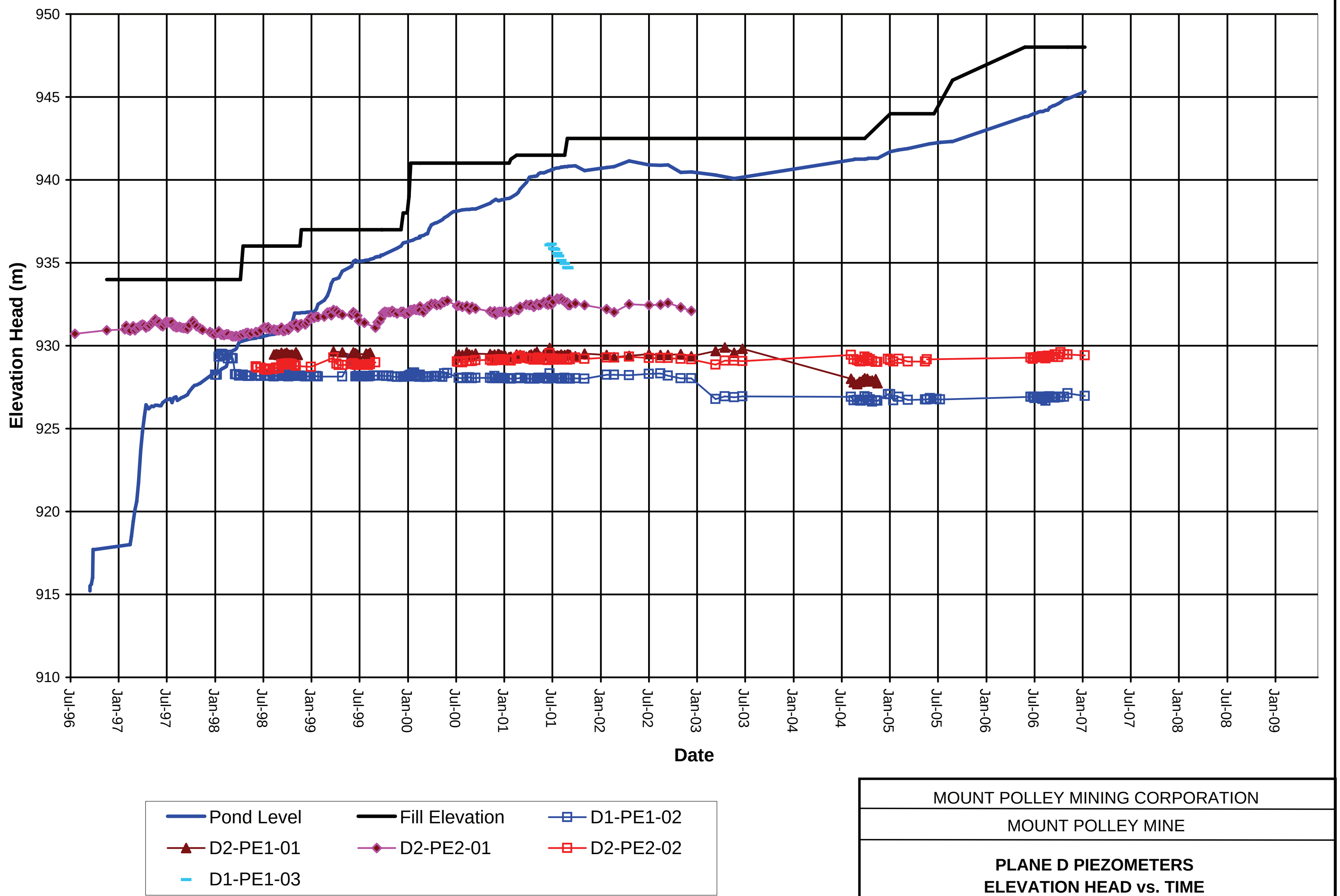
MOUNT POLLEY MINING CORPORATION		
MOUNT POLLEY MINE		
PLANE C PIEZOMETERS ELEVATION HEAD vs. TIME		
Knight Piésold CONSULTING	PROJECT NO. VA 101-1/10	REF. NO. 1	REV 0
	FIGURE 2.10	



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MOUNT POLLEY MINING CORPORATION			
MOUNT POLLEY MINE			
PLANE D PIEZOMETERS **ELEVATION HEAD vs. TIME**			
Knight Piésold CONSULTING	PROJECT NO. VA 101-1/10	REF. NO. 1	REV. 0
	FIGURE 2.11		

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MOUNT POLLEY MINING CORPORATION			
MOUNT POLLEY MINE			
PLANE E PIEZOMETERS ELEVATION HEAD vs. TIME			
Knight Piésold CONSULTING	PROJECT NO. VA101 - /10	REF. NO. 1	REV. 0
	FIGURE 2.12		

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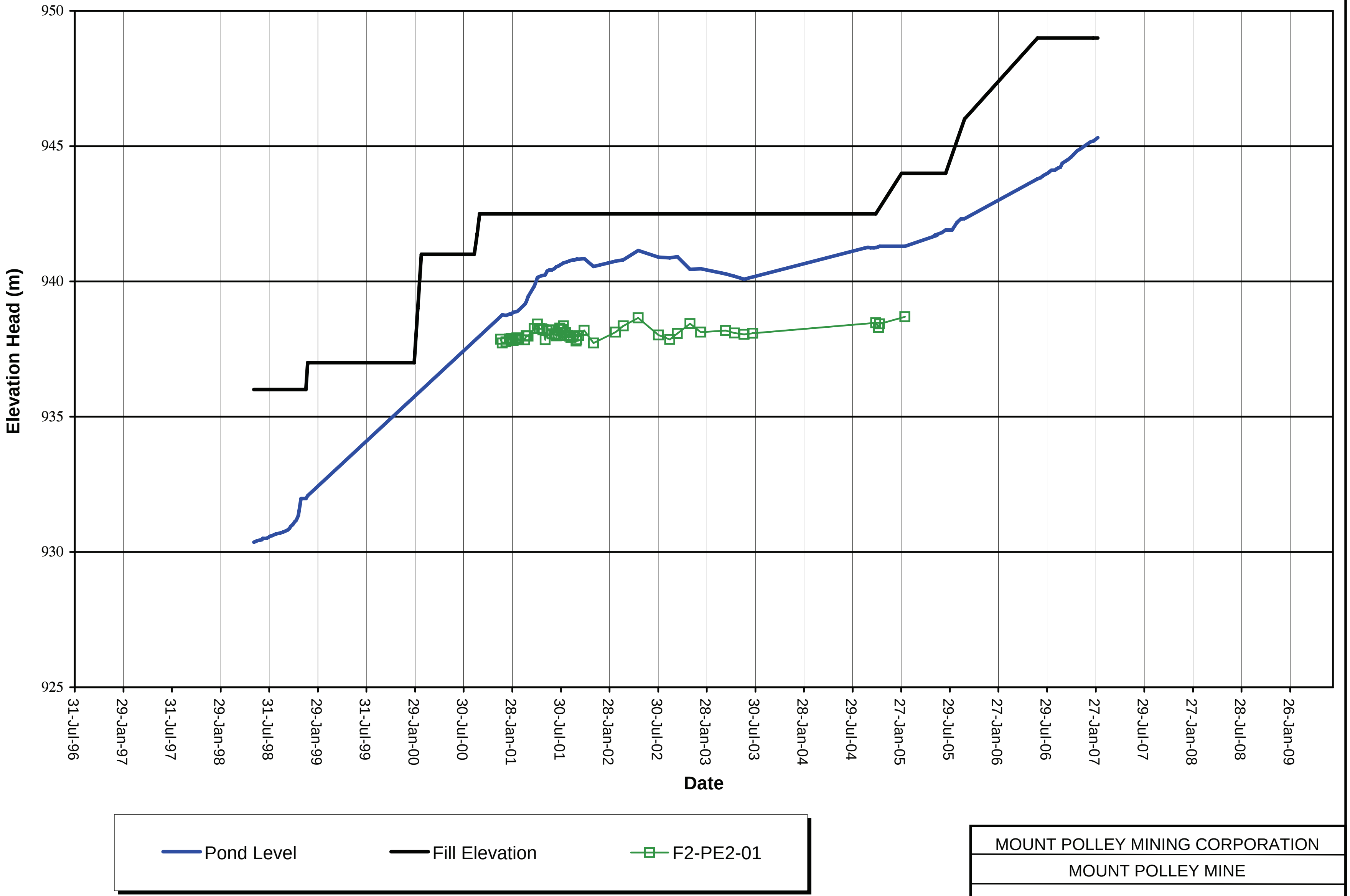

MOUNT POLLEY MINING CORPORATION

MOUNT POLLEY MINE

PLANE F PIEZOMETERS
ELEVATION HEAD vs. TIME

Knight Piésold
CONSULTING

PROJECT	REF.	RE
VA 101-1/10	1	0

FIGURE 2.13

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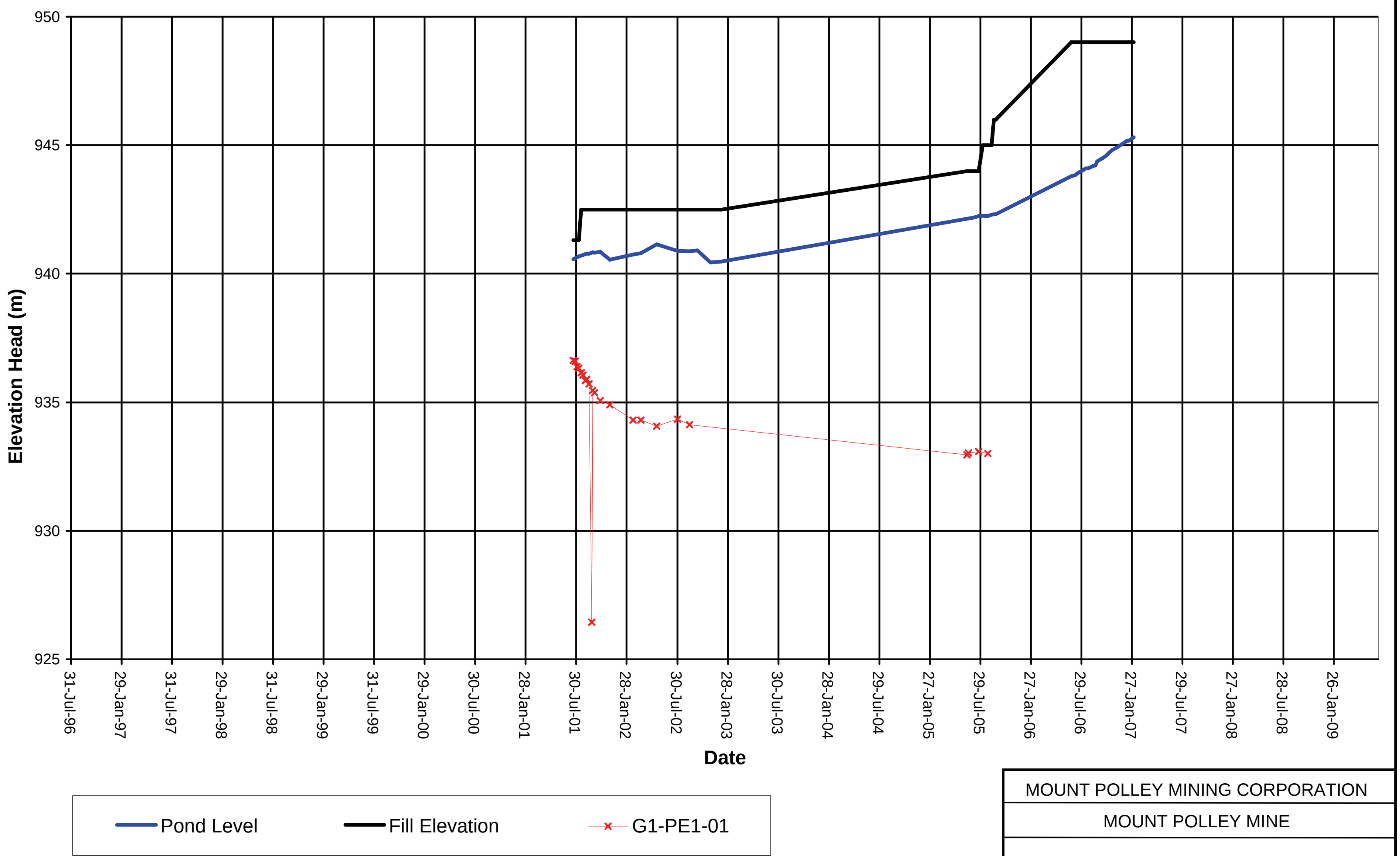

MOUNT POLLEY MINING CORPORATION		
MOUNT POLLEY MINE		
PLANE G PIEZOMETERS ELEVATION HEAD vs. TIME		
PROJECT	REF.	REV.
VA101-1/10	1	0
FIGURE 2.14		

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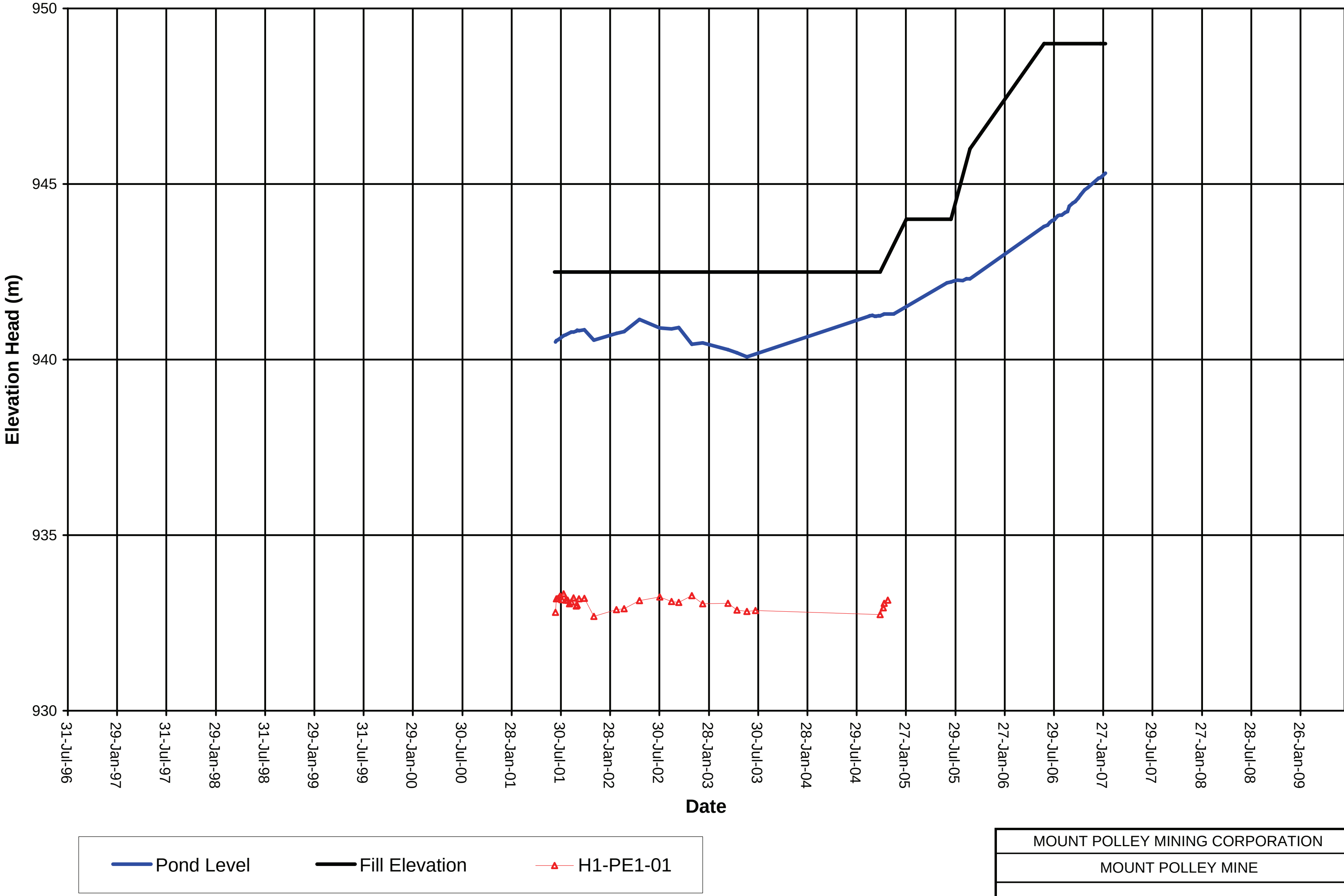
AMEC010469_0033

Revised March 7, 2007



MOUNT POLLEY MINING CORPORATION		
MOUNT POLLEY MINE		
PLANE H PIEZOMETERS ELEVATION HEAD vs. TIME		
Knight Piésold CONSULTING	PROJECT / ASSIGNMENT NO. VA101-1/10	REF NO. 1
	FIGURE 2.15	REV. 0

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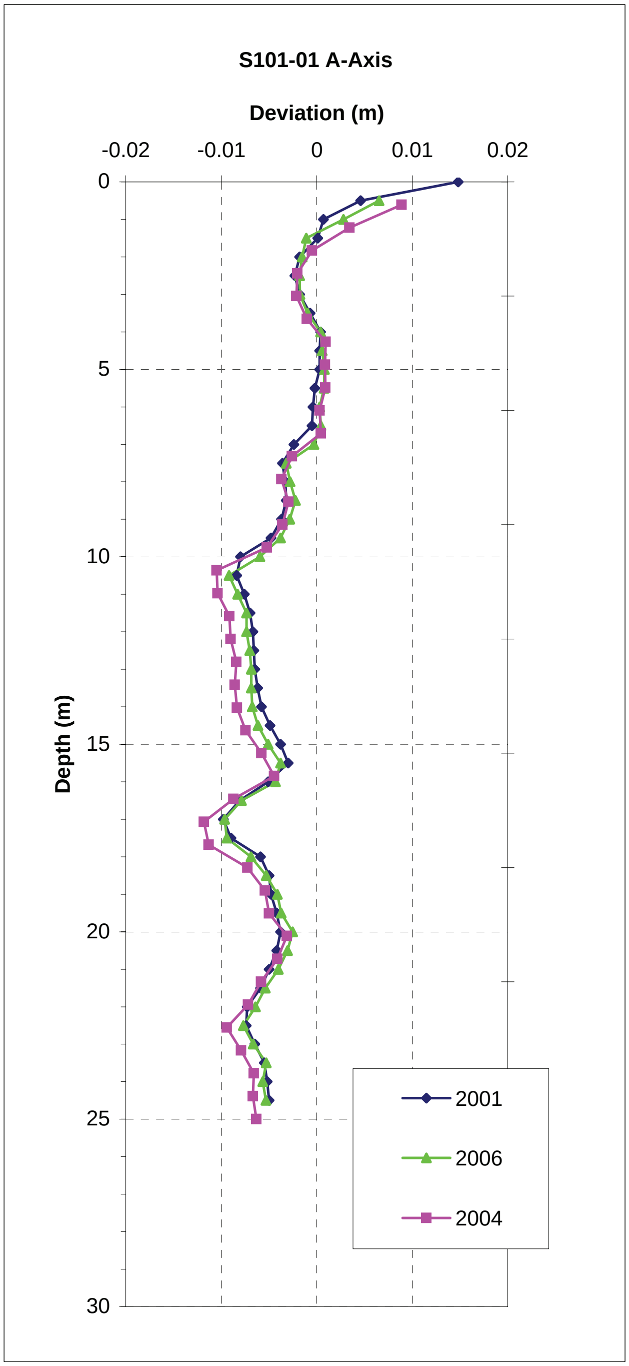

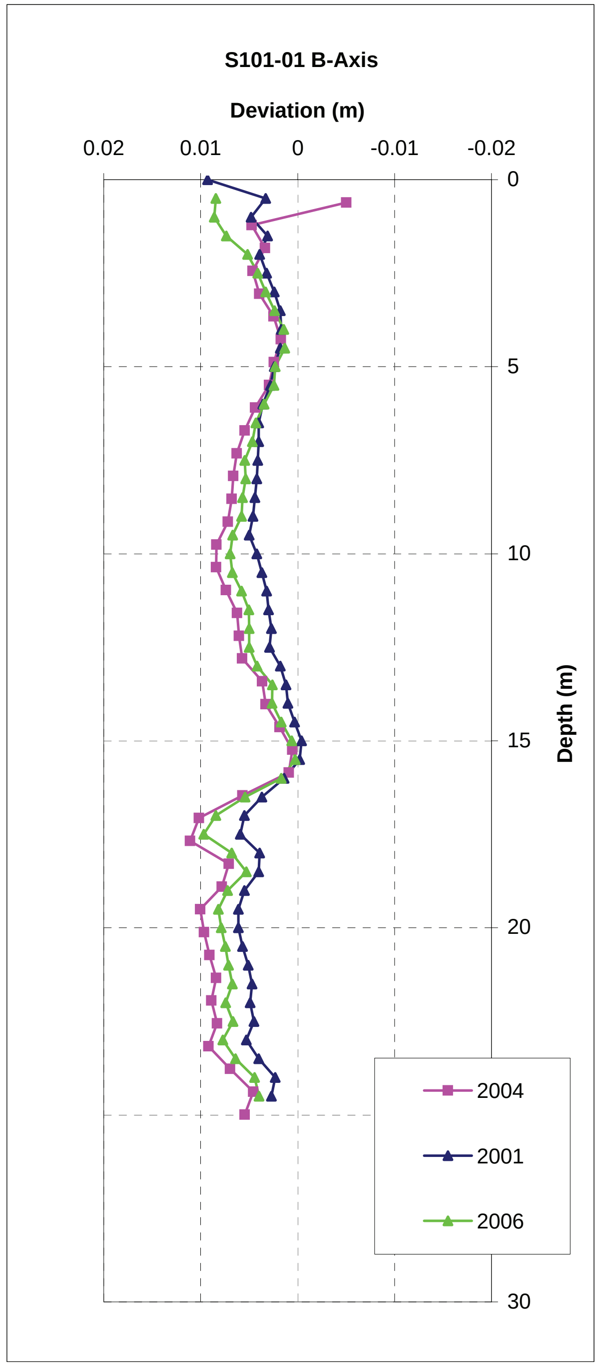

SI01-01 no longer functioning.

MOUNT POLLEY MINING CORPORATION

MOUNT POLLEY MINE

DOWN HOLE INCLINOMETER DISPLACEMENT
SI01-01

Knight Piésold
CONSULTING

PROJECT / ASSIGNMENT NO.	REF NO.
VA101-1/10	1

FIGURE 2.16 REV. 0

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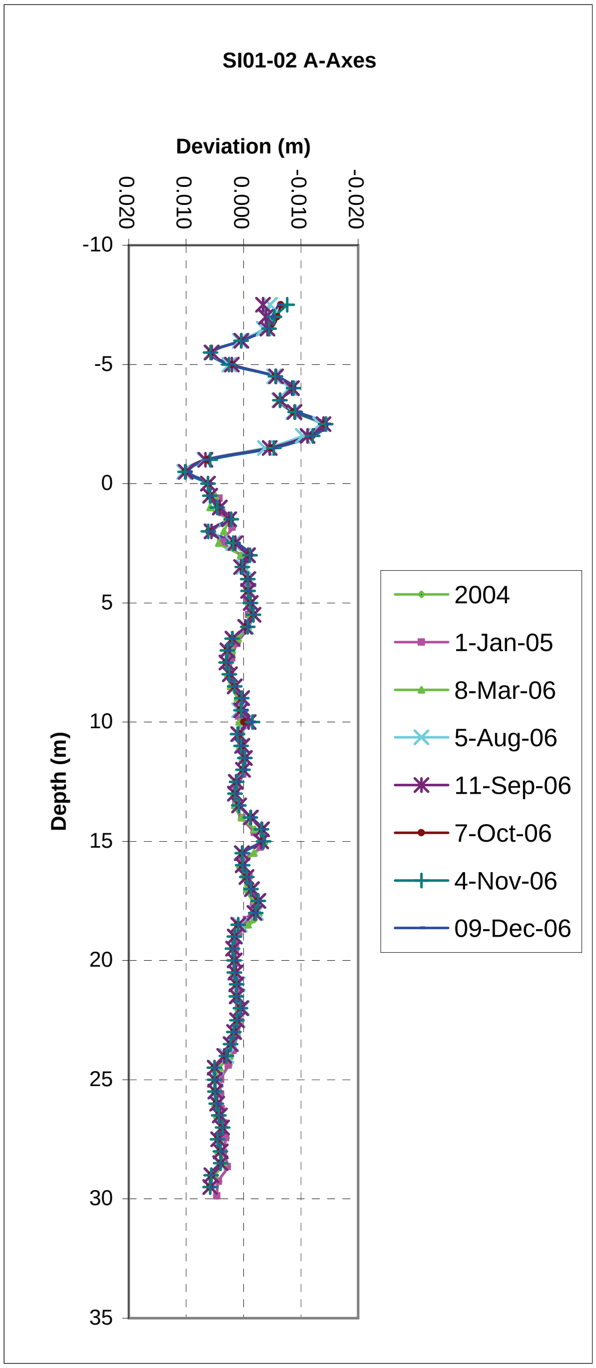

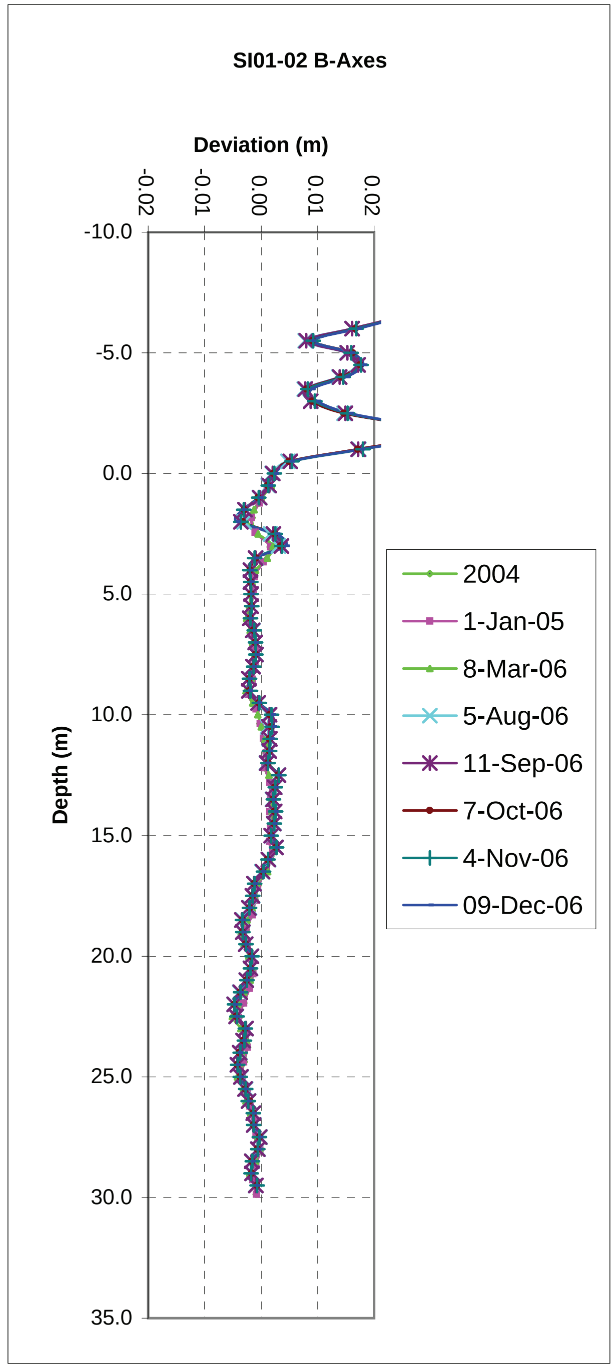

MOUNT POLLEY MINING CORPORATION

MOUNT POLLEY MINE

DOWN HOLE INCLINOMETER DISPLACEMENT
SI01-02

Knight Piésold
CONSULTING

PROJECT / ASSIGNMENT NO.	REF NO.
VA101-1/10	1

FIGURE 2.17 REV. 0

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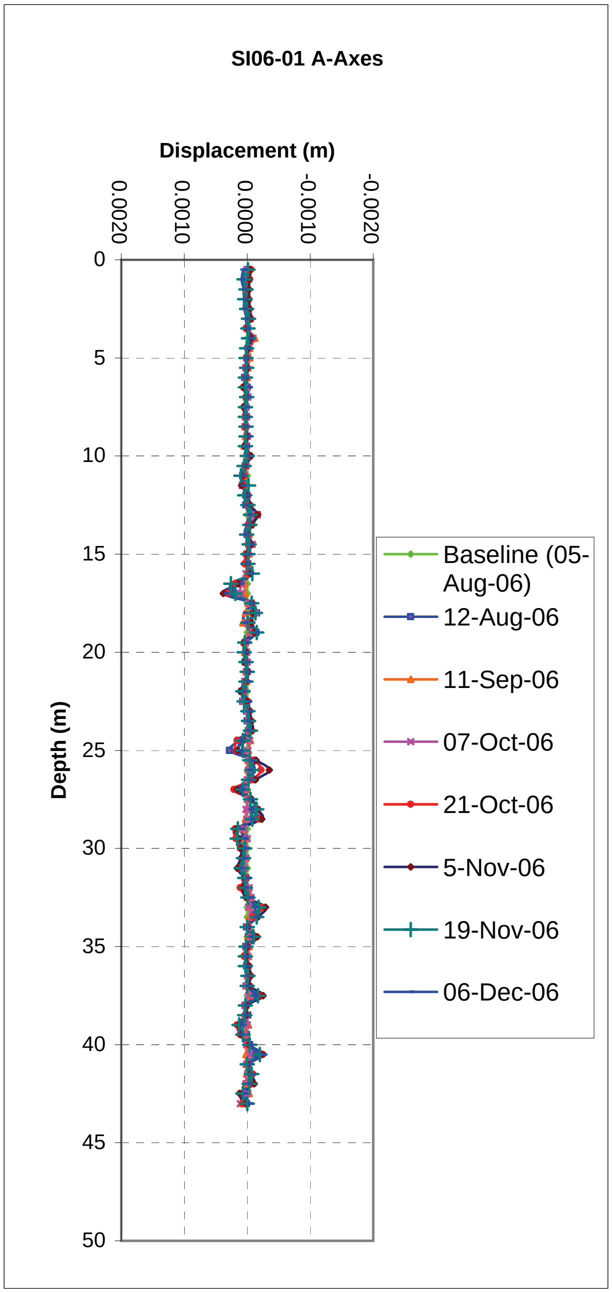

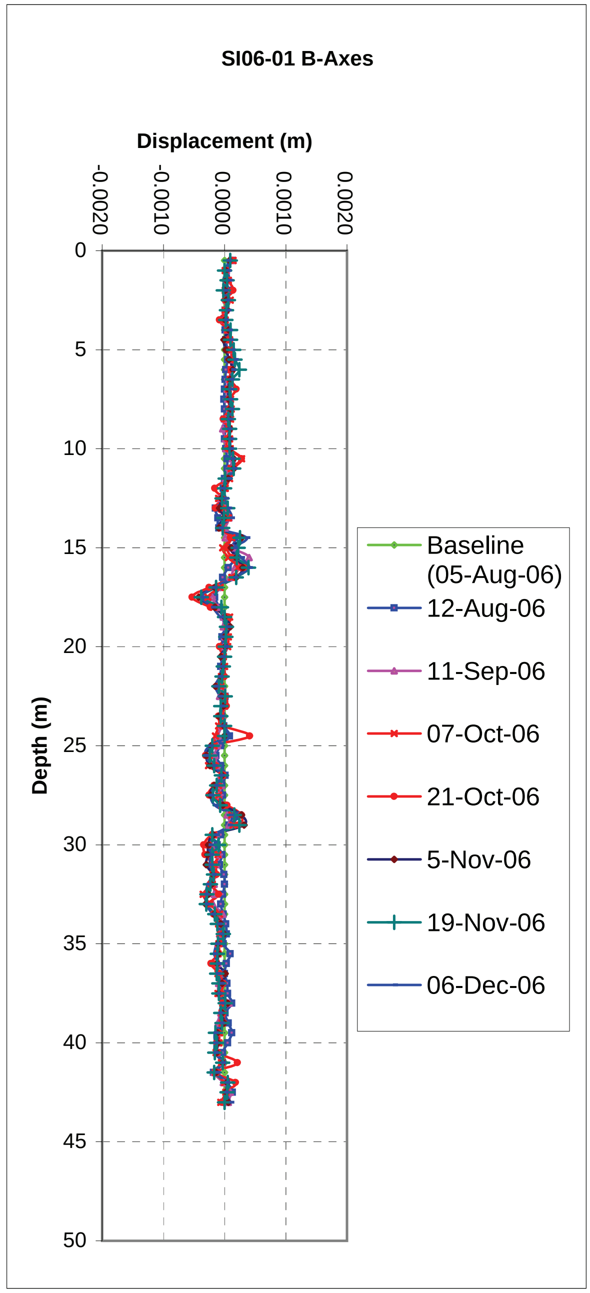

MOUNT POLLEY MINING CORPORATION

MOUNT POLLEY MINE

DOWN HOLE INCLINOMETER DISPLACEMENT SI06-01

Knight Piésold CONSULTING

PROJECT / ASSIGNMENT NO.	REF NO.
VA101-1/10	1

FIGURE 2.18 REV. 0

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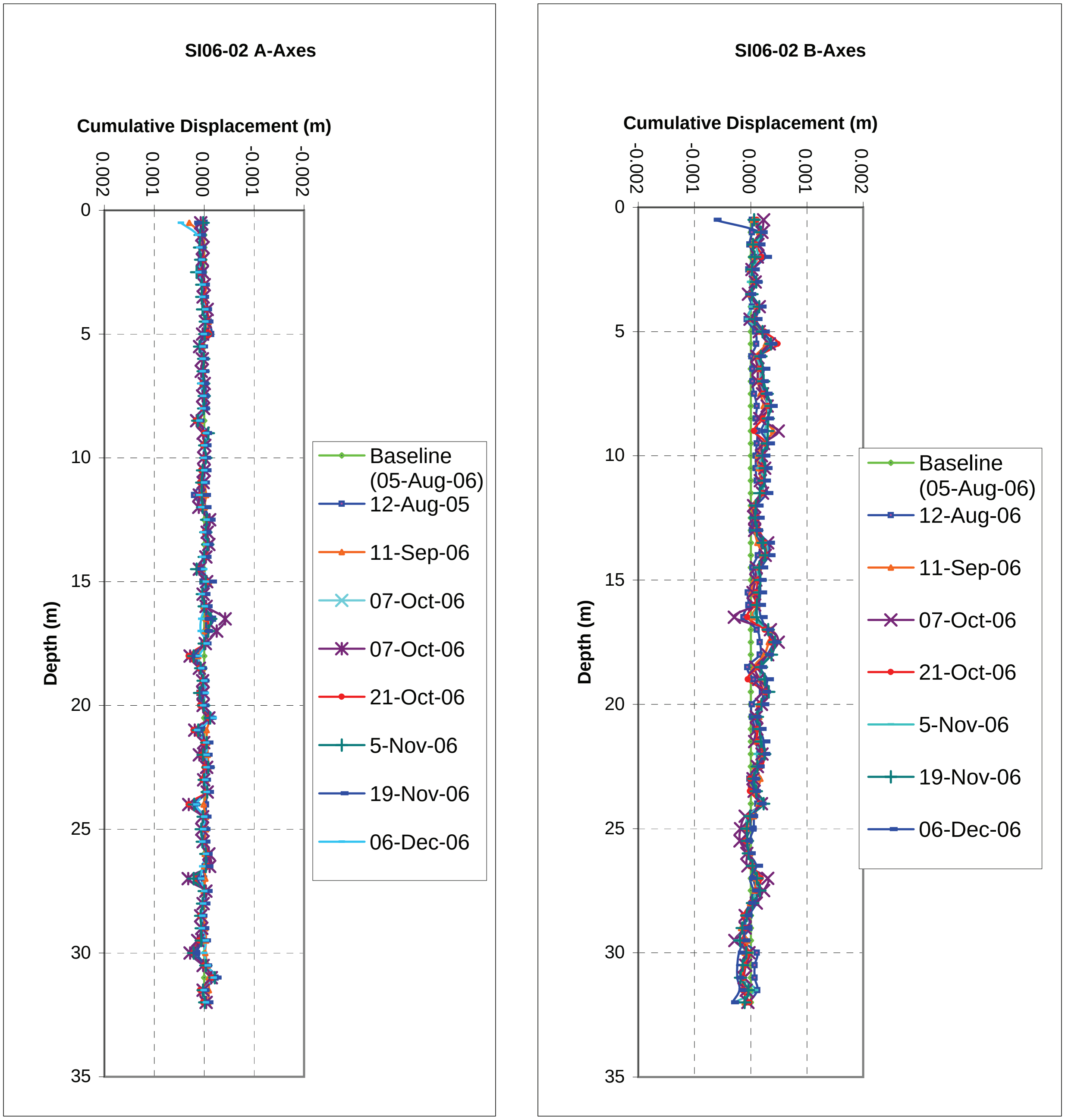

MOUNT POLLEY MINING CORPORATION	
MOUNT POLLEY MINE	
DOWN HOLE INCLINOMETER DISPLACEMENT SI06-02	
PROJECT/ASSIGNMENT NO. VA101-1/10	REF NO. 1
FIGURE 2.19	REV. 0

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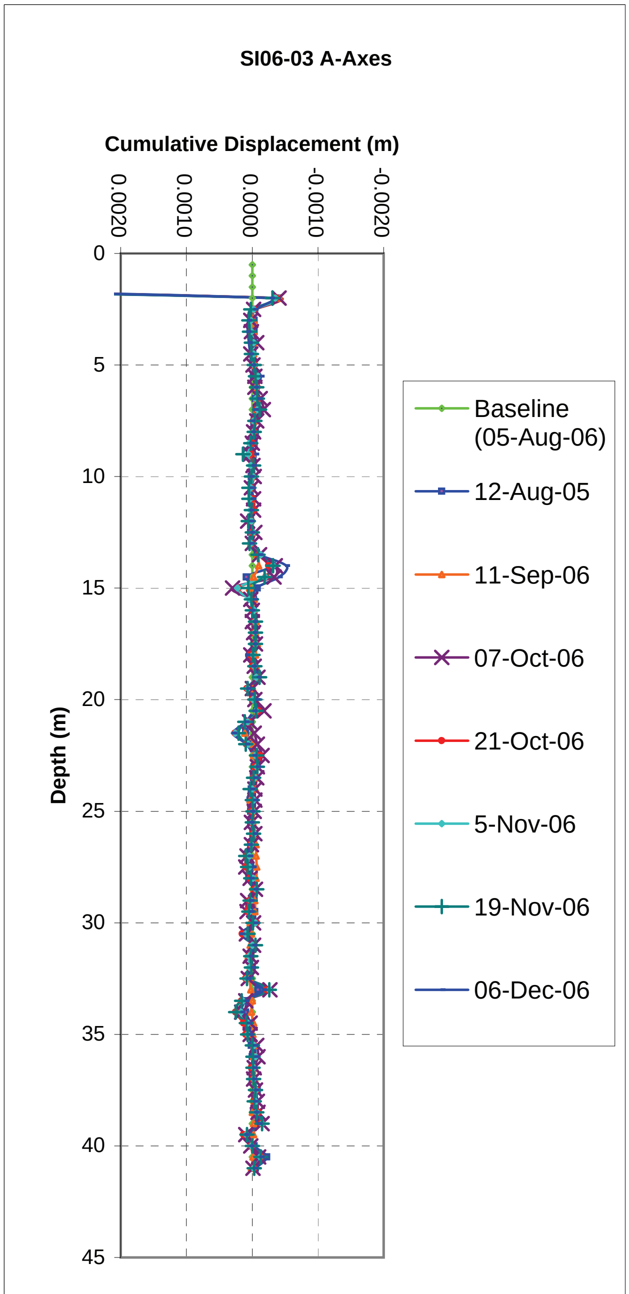

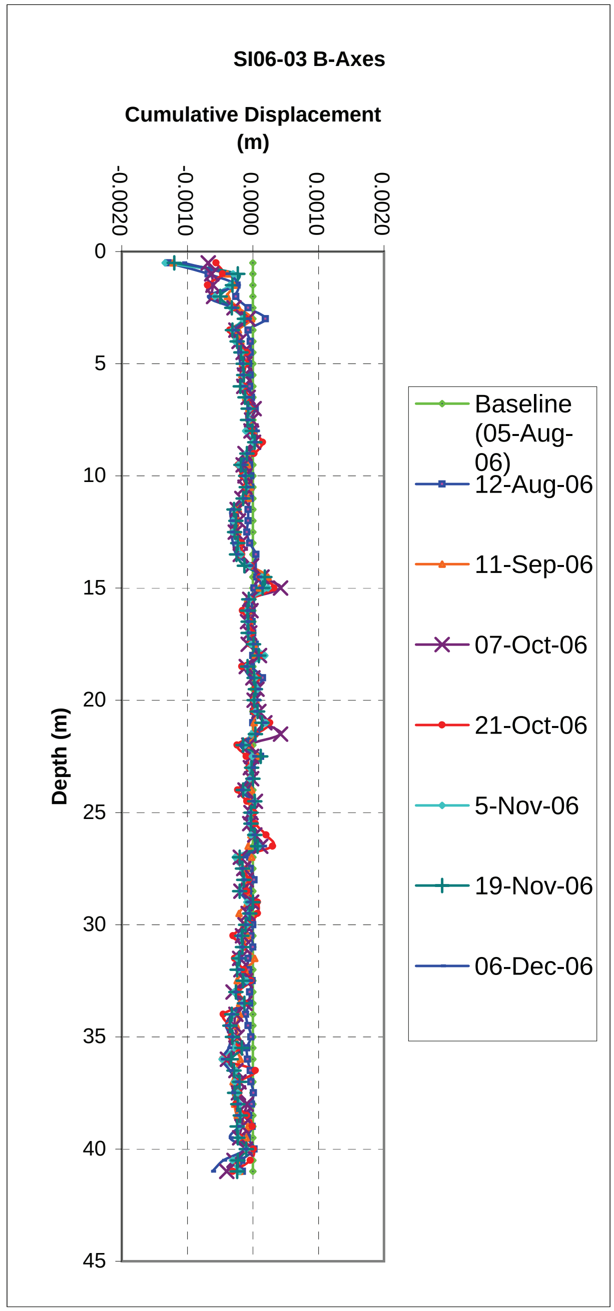

MOUNT POLLEY MINING CORPORATION		
MOUNT POLLEY MINE		
DOWN HOLE INCLINOMETER DISPLACEMENT SI06-03		
Knight Piésold CONSULTING	PROJECT/ASSIGNMENT NO. VA101-1/10	REF NO. 1
	FIGURE 2.20	REV. 0

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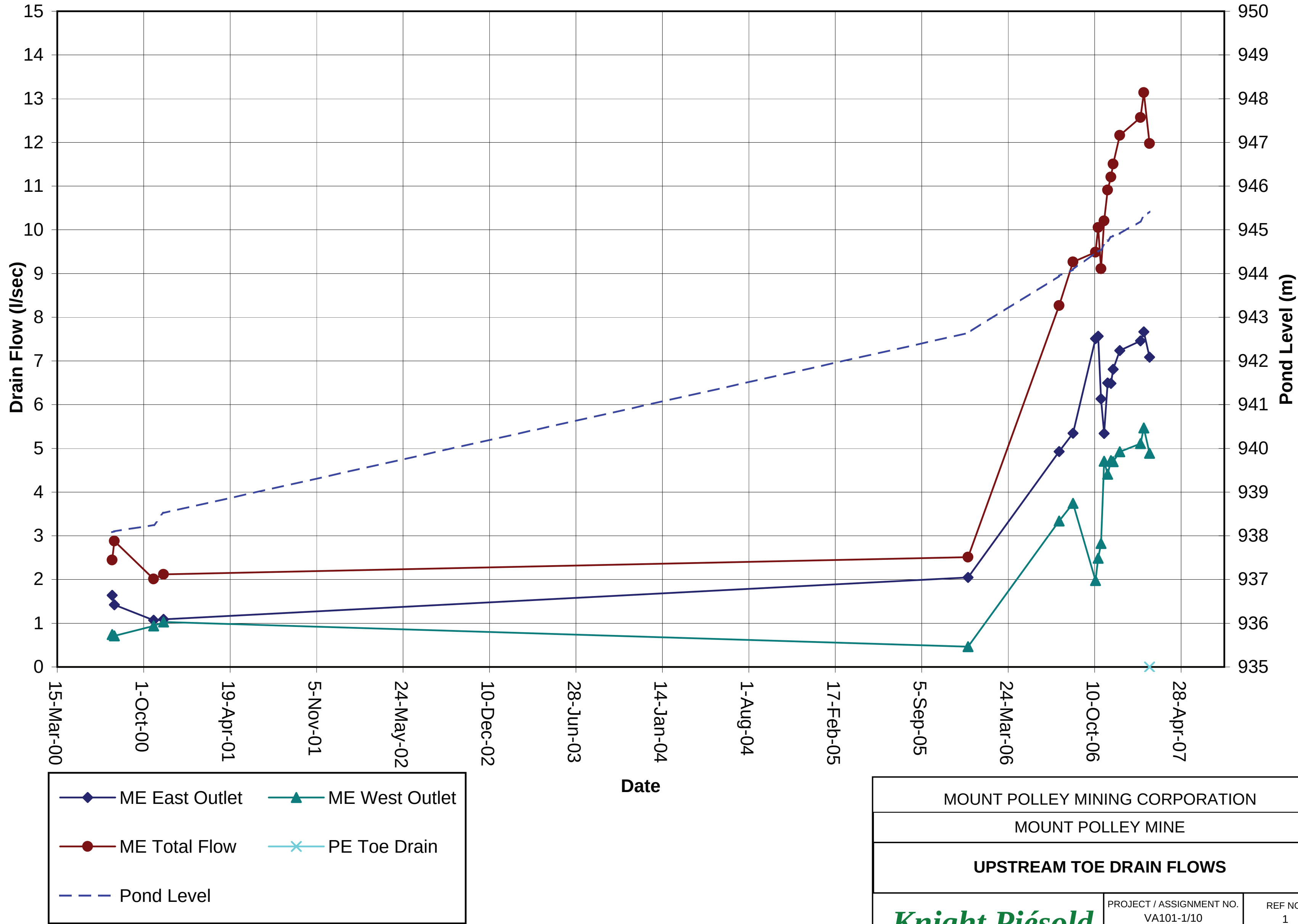

MOUNT POLLEY MINING CORPORATION

MOUNT POLLEY MINE

UPSTREAM TOE DRAIN FLOWS

Knight Piésold CONSULTING

PROJECT / ASSIGNMENT NO. VA101-1/10

REF NO. 1

FIGURE 2.21

REV. 0

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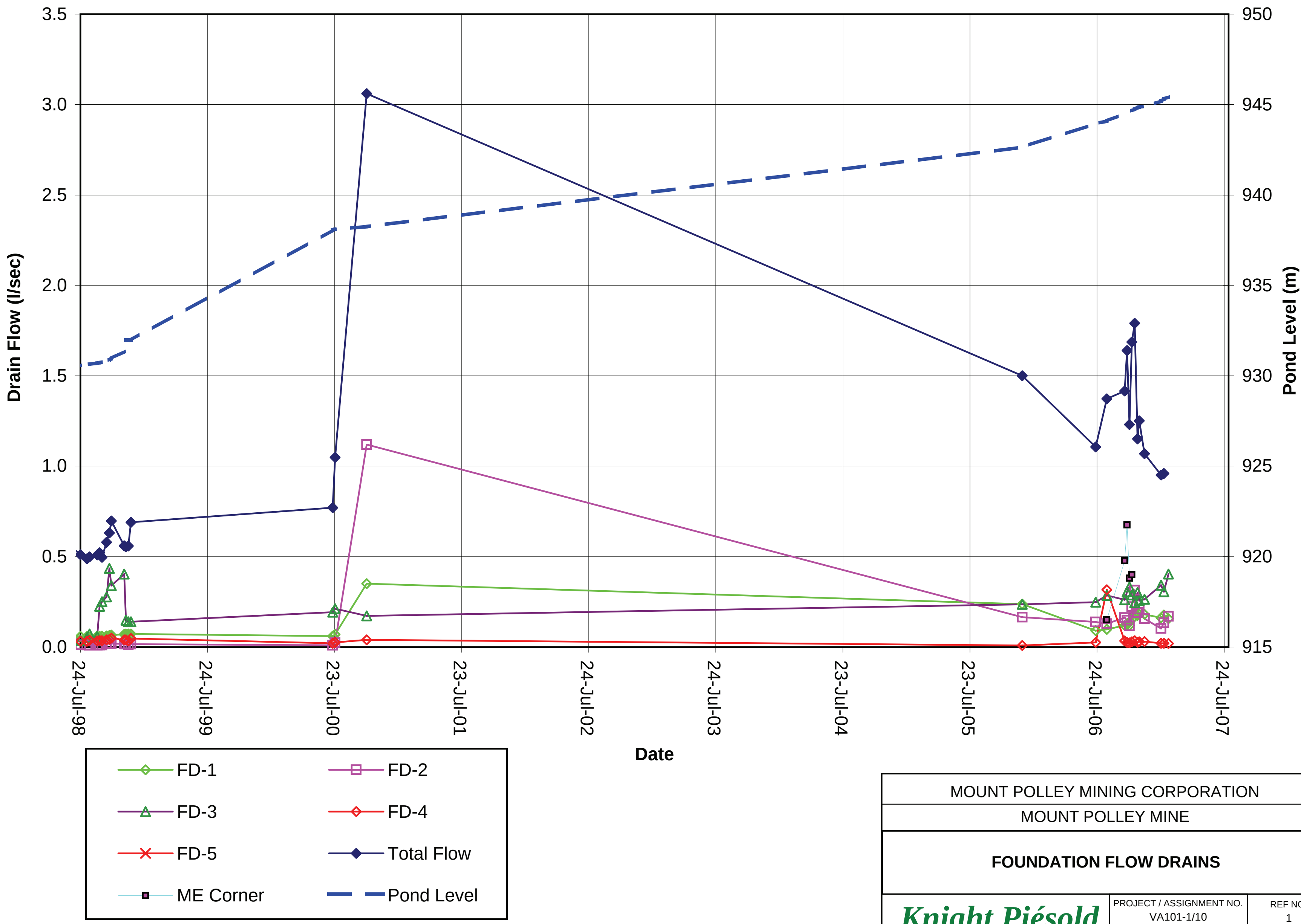

MOUNT POLLEY MINING CORPORATION		
MOUNT POLLEY MINE		
FOUNDATION FLOW DRAINS		
Knight Piésold CONSULTING	PROJECT / ASSIGNMENT NO. VA101-1/10	REF NO. 1
	FIGURE 2.22	REV. 0

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